

MAERSK BELCON LOGISTICS HUB



STORMWATER MANAGEMENT PLAN

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Prepared for:

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1. INTRODUCTION

KLS Consulting Engineers has been appointed by Maersk Logistics & Services South Africa (Pty) Ltd and Profica (Pty) Ltd as the Civil Engineering consultants for the proposed BELCON Logistics hub in Bellville, Cape Town.

The development is classified as a Brownfield development, with a total size of 11 ha. The purpose of this report is to address the stormwater management strategy for this development, as well as all secondary stormwater related issues that may arise post-development.

The KLS-strategy for the effective management of stormwater, is to implement a sustainable urban drainage system for the effective reduction and treatment of all runoff generated from the post development site, to ensure the protection of downstream infrastructure, watercourses, and ecosystems against events of abnormally high rainfall. The internal stormwater system will comply with City of Cape Town (*hereafter referenced as CCT*) regulations and policies, as given in the following documents:

- CCT: Management of Urban Stormwater Impacts Policy (27 May 2009).
- CCT: Floodplain and River Corridor Management Policy (27 May 2009).
- CCT: Stormwater Management By-Law (30 August 2005).
- The Roads Drainage Manual (South African National Roads Agency Limited, 2006).
- The Georgia Stormwater Management Manual, Volume 2.
- The Neighbourhood Planning and Design Guide (2019).
- CCT: Standards and Guidelines for Roads and Stormwater, Version 3 (2022).

A preliminary meeting was held on 23rd of February 2023 at the CCT offices in Bellville, to engage with the CCT *officials* prior to submitting detailed plans and designs for approval and to determine if there are any major development constraints. CCT confirmed they agree with the KLS *approach* and can proceed with detail design.

Further guidelines and references used in the design of the internal stormwater network and the compilation of this report, is as follow:

- Topographical Survey of the Site (Joubert & Brink Surveys, 2022)
- Belcon Logistics Hub Site Plan (Anderson Perry Partnership Architect's, 2023)
- Geotechnical Investigation (SRK Consulting, 2023)
- Belcon Logistics Hub Landscape Plan (CNDV Landscape Architects, 2023)

2. DESCRIPTION OF THE SITE

The 11ha development is situated on Erf 14876, Erf 14777, Erf 14878, Remainder of. Erf 14867, Erf 12873, and Erf 1488. It is located within Transnet Park, along Robert Sobukwe Road. The property is located between the two entrances in Mc Donald and Belcon Road, respectively. See **Figure 1**

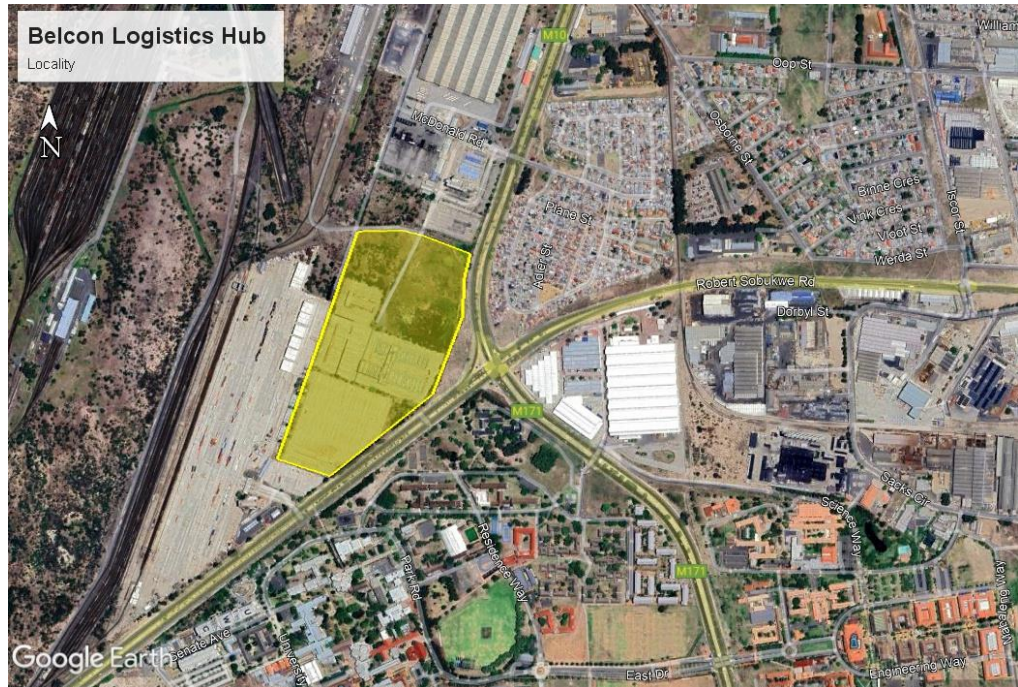


Figure 1: Belcon Logistics Hub Locality

2.1. EXISTING TOPOGRAPHY

The existing development site is flat, with no significant fall in one direction. The site has been developed, with slopes across the existing hardstands. The overall slope falls towards the North East, with heights varying from approximately 58 to 60 MSL.

Refer to **Appendix A** for the pre-development Topographical Survey.

2.2. CURRENT SITE COVERAGE

The proposed development would be constructed on existing marshalling yards that were constructed for Transnet in the 1980s.

The proposed area has numerous small structures and buildings. The main two buildings are the Motor Technical Vehicle (MTV) Workshop and the Paint Shop and Wash Bay.

The buildings are surrounded by concrete hardstands; however, the northern part of the proposed development has not been developed.

Well-established trees and vegetational growth are present in the undeveloped areas.

The site consists of approximately 66000 m² of permeable undeveloped areas, and 44000 m² of developed, impermeable surfaces.

2.3. CLIMATE

The development site falls within a winter rainfall region, with an average rainfall of 506 mm per annum. **Table 1** gives a summary of the rainfall data used for the internal stormwater network design, as obtained from the City of Cape Town Rainfall data, which includes a 15% increase for climate change.

Rainfall Data - Belcon Logistics Hub								
Reference Coordinates		33.93°, 18.63°						
Coordinates in (Degrees, minutes)		33°55', 18°37'						
MAP (mm)		506						
Return Period	0.5	1	2	5	10	20	50	100
1-day Precipitation depth (mm)			39.3	52.8	62.6	72.7	87.1	98.8
1440-min precipitation depth (mm)	19.5	32.0	44.5	61.0	73.5	86.0	102.5	115.0
30-min precipitation depth (mm)			12.2	16.3	19.4	22.5	27.0	30.7

Table 1: Rainfall data from CCT Climate Change Rainfall Grid

2.4. PROPOSED DEVELOPMENT

The proposed development is a logistics hub, which is primarily a new container depot facility. The following amenities are included:

- Relocated Container wash-bay facility.
- Container repair workshop.
- Warehouse facility for ambient and cold storage goods.
- Office building.
- Guardhouses.
- Marshalling yards.

Figure 2 indicates the site layout.

For the detailed plan refer to **Appendix B**

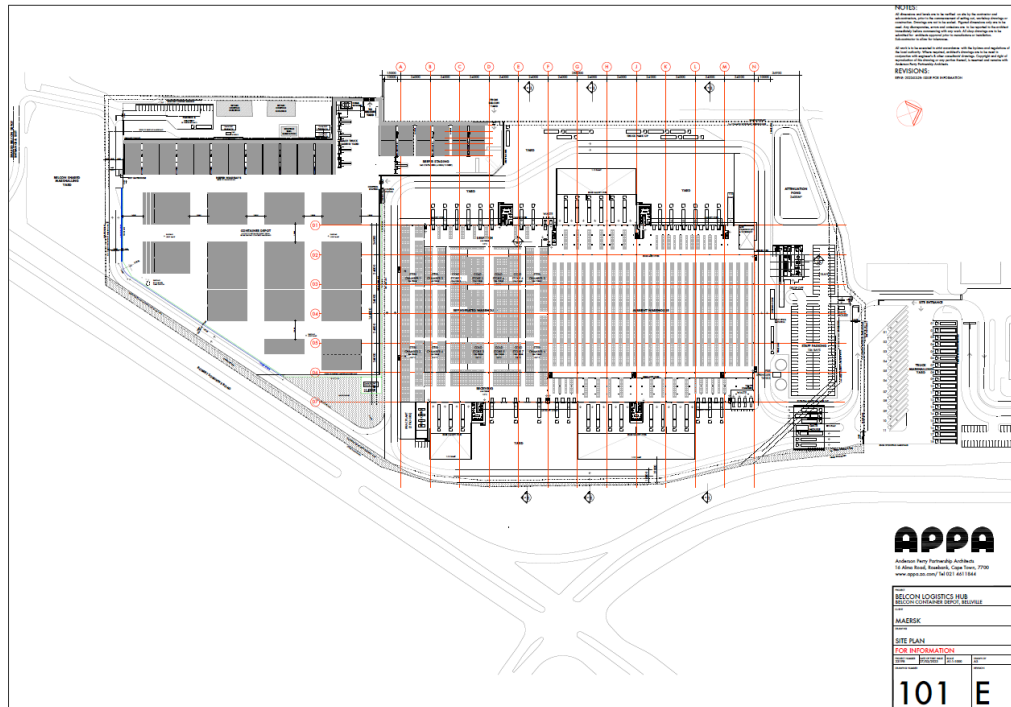


Figure 2: Belcon Logistics Hub Site Layout

2.5. GEOTECHNICAL INVESTIGATION

SRK Consulting South Africa (Pty) Ltd. was appointed by Maersk Logistics & Services South Africa (Pty) Ltd. to undertake a detailed geotechnical investigation of the proposed development site. **The geotechnical investigation was done on 23 May 2023.** For this report, 10 test pits were excavated at selected positions to cover both the Greenfields part of the site as well as the portion occupied by existing warehouses, with Dynamic Probe Light (DPL) penetrometer tests conducted at the locations of the test pits, indicated on **Figure 3. Table 2** gives a summary of the water table levels at each test pit. The attenuation facility is proposed at the location of test pit 7, at which no seepage was found up to 2.6 m.

Test Pit	Rest Water Table Level
TP1	1.2 m
TP2	1.3 m
TP3	1.6 m
TP4	1.4 m
TP5	1.2 m
TP6	1.6 m
TP7	No seepage up to 2.6 m
TP8	1.6 m
TP9	1.4 m
TP10	1.4 m

Table 2: Water Table Depth At Test Pit Locations

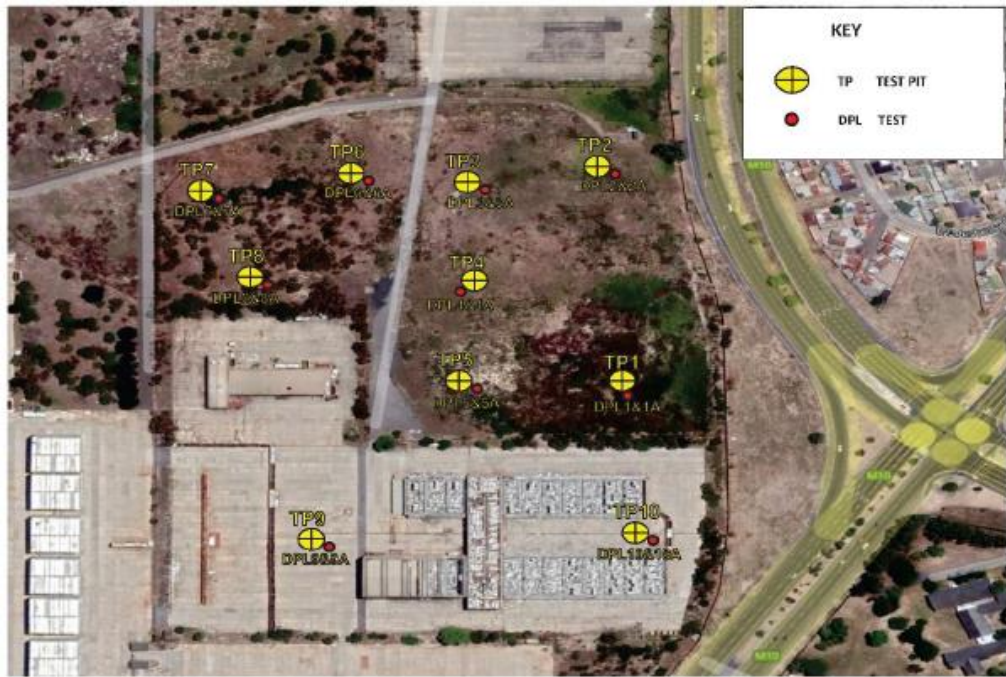


Figure 3: SRK Test Pit Locations

The region's published geological map shows deep weathered Malmesbury Group shale, and greywacke beneath more recent Quaternary layers. The test pits excavated at the site indicate a similar soil profile. A thin layer, varying in thickness from about 0.3 m to 0.9 m thick, of sandy/gravelly fill is present. The upper 300 mm soil profile contains abundant roots that should be removed or stockpiled. The fill is underlain by naturally transported, fine to medium sand which is generally of loose consistency and tends to improve to medium-dense consistency with increasing depth.

In addition to the on-site investigations and testing, soil samples were submitted for laboratory testing. The grading and Atterberg Limits indicate that the underlying transported soils are made up primarily of fine to medium sand with a total clay and silt concentration of less than 4%. The sand is granular non-cohesive with zero PI and has no expansion potential. Refer to **Annexure A** for the full Geotechnical Report

3. STORMWATER MANAGEMENT

The stormwater network is designed to manage post-development runoff for all minor and major storm events, in a sustainable manner, while simultaneously adhering to all authoritative requirements and conditions of approval as set out by the City of Cape Town. Refer to correspondence from the relevant CCT departments in **Appendix C** for the conditions and requirements to be included in the stormwater design and management plan.

The internal network consists of underground gravity fed stormwater box culverts and pipes, ACO Q-max drains, a series of inlet structures, and

open swales, which will drain the impermeable surfaces. All runoff will ultimately be conveyed into a newly constructed attenuation pond, which will discharge runoff from the site at pre-development rates.

The following minimum specifications, as per the City of Cape Town Municipal Guidelines, will be implemented for the underground stormwater infrastructure:

- Concrete spigot and socket class 100D pipes will be installed.
- A minimum pipe diameter of 300 mm for connections, and 375 mm for main lines is used.
- A Minimum velocity of 0.9 m/s is ensured in the design.
- A Maximum spacing of 90 m is adhered to.
- All stormwater structures will be constructed according to the CCT standard details.

The following assumptions and design principles are used in the stormwater management design:

- The rational formula is used for peak flow calculations for all design storm events:

$$Q = \frac{C \times I \times A}{3600}$$

- The run-off coefficient, C, is calculated as per the SANRAL Drainage Manual.
- Intensities used for design floods, are the average of the Rational Method (Alternatives 1, 2 and 3) and the SDF Method, as per the SANRAL Drainage Manual.
- Attenuation and treatment volumes are calculated as per the Georgia Stormwater Management Manual Water Quality Volume formula, which is the preferred CCT-formula:

$$WQv = \frac{P \times Rv \times A}{1000}$$

- Precipitation depth (P) used in the Rational Method, Alternative 3 and Water Quality volume calculations, is obtained from the CCT Design Rainfall Grid (2013).
- The underground stormwater network is designed to convey runoff of up to the 1:10 -year storm event to the attenuation pond.

- The attenuation pond is designed as a wet pond, to ensure that sustainable urban drainage system (SUDS) and LEED-certification requirements are met. The aim is to achieve LEED platinum accreditation.

The KLS approach for stormwater management, which will be discussed in this section, aims to:

- Keep natural drainage patterns in the post development site, where possible, within the constraints of the new developed site.
- Protect adjacent and downstream properties, water courses and infrastructure from the impacts of increased post development runoff.
- Improve the quality of runoff by reducing the total amount of suspended solids, nitrogen and phosphorus content.
- Promote infiltration and internal reuse of runoff.
- Implement best management practices for a water sensitive urban design and sustainable stormwater management.

For full stormwater calculations, refer to **Appendix D**.

3.1. EXISTING STORMWATER NETWORK

The existing stormwater network consists of an underground gravity pipe network and inlet structures, that drain the impermeable areas. This system is designed to have sufficient capacity to convey a 1:5-year rainfall event.

3.2. MANAGING RUNOFF FROM ADJACENT PROPERTIES

CCT indicated that flooding occurs in Parow Industrial, north-west of the proposed development due to excessive stormwater runoff from Transnet Park.

The proposed solution from CCT (see correspondence in **Appendix C2**), was to install a cut-off drain along the railway lines and tying it into an existing earth drain as indicated in **Figure 4**. GIBB consulting Engineers analysed the stormwater runoff for CCT and this led to the size of the cut-off drain to be 0.5m deep, 0.5 m wide with side slopes of 1:3 at the start. The cut-off drain increases to 1.2 m deep, 1.2 m wide with side slopes of 1:3 at the end.

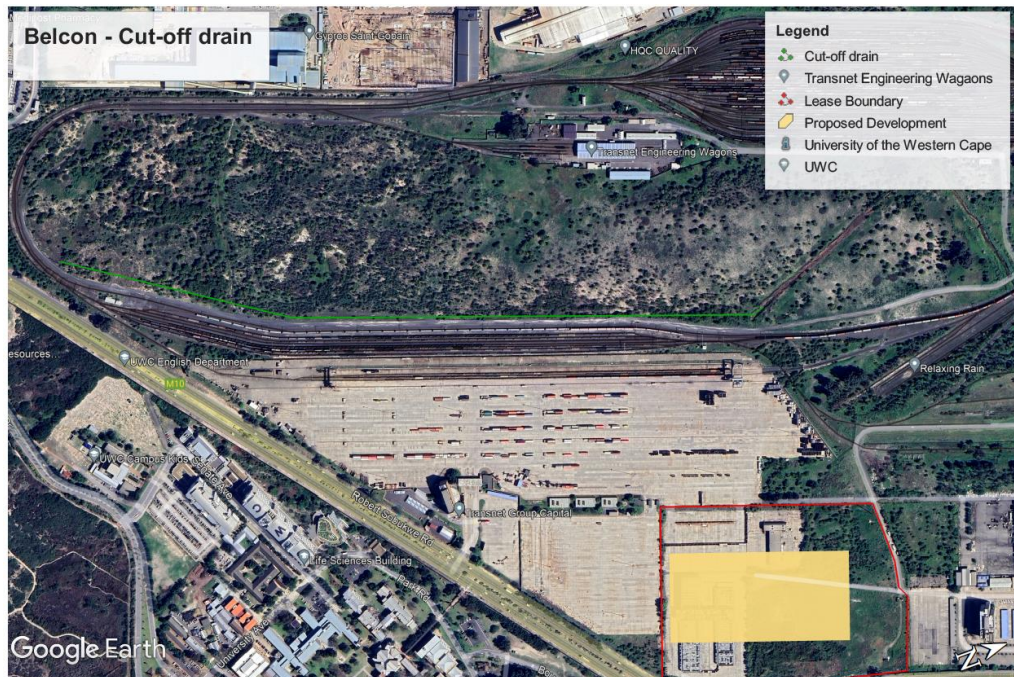


Figure 4: Cut-off drain position.

KLS will incorporate the cut-off drain into the works as this will be a condition with the SDP approval.

As per CCT instruction, a low point was identified in Robert Sobukwe Road, just North of the development site. During events of extreme rainfall, flooding may occur and runoff reaching the development site is a possibility, due to the natural overland flow routes. The post development site topography will convey runoff due to possible flooding towards the internal swale network, which channels runoff towards the attenuation pond.

The warehouse is ± 1.4 m higher than the marshalling yards, therefore a very low risk of flooding of the buildings exists.

3.3. NEW INTERNAL NETWORK

The proposed development will increase the existing impervious cover to approximately 85%, which will result in an increase in stormwater runoff. The new development will drain the impervious areas via an underground network, consisting of the following elements:

- ACO Q-Max drains,
- ACO Q-Max junction boxes,
- Box culverts,
- Concrete spigot and socket pipes.
- Grids and side inlet structures.
- uPVC pipes class 34, heavy duty.

Refer to **Appendix E** for the proposed stormwater layout plan.

The development is divided into three main sub-catchments contributing to the attenuation facility. Catchment A covers the majority of the site. Catchment B and C drains towards a series of swales to the north of the site, which ultimately discharges into the attenuation dam in the northwestern corner of the site. Refer to **Figure 5** for the sub catchments and overland flow routes.

The stormwater network has capacity to effectively convey all runoff for up to the 1:10 year rainfall event to the attenuation facility in the northwestern corner of the site, before discharging into the municipal system via 450 mm \varnothing concrete pipe.

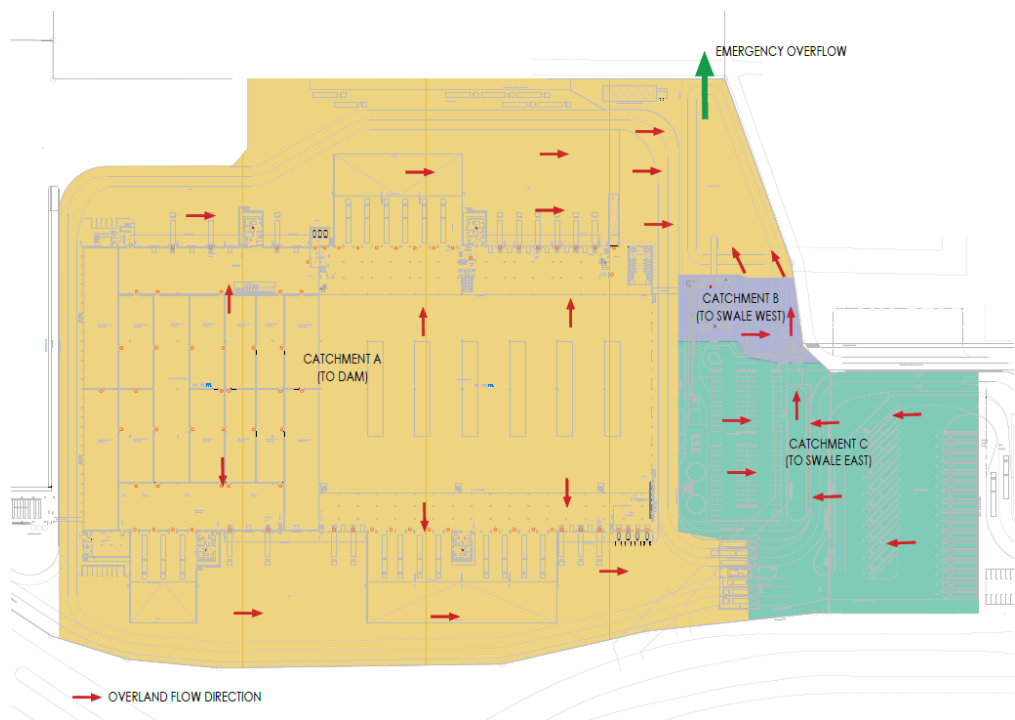


Figure 5: BELCON development sub-catchments and overland flow routes

3.4. ATTENUATION

The City of Cape Town Management of Urban Stormwater Impacts Policy requires that the stormwater infrastructure makes allowance for the attenuation of peak runoff flows to pre-development runoff rates, in order to protect downstream watercourses and infrastructure.

A stormwater attenuation dam will be constructed in the northwestern corner of the site. (see **Appendix F1** for the Dam layout and sections) The dam is designed as a wet pond with no engineered lining at this

stage, due to a high water table. The dam will have a permanent water level (PWL) of 1.6 m depth, to ensure no reed growth within the dam. The volume of 2156.6 m³ at PWL is more than the required water quality volume (see section 3.5). The dam is designed with additional storage capacity above the PWL, to attenuate runoff from up to a 1:100-year peak storm rainfall event. This also meets the CCT Management of Urban Stormwater Impacts Policy required storage capacity for a volume equal to the 1:1-year, 24-hour storm event, which amounts to 2878 m³.

The runoff volume generated by the 1:1-year, 24-hour storm event will be attenuated and released into the municipal system over 26.33 hours. See **Figure 6** for the graphical representation of the cumulative runoff volume generated, the total volume discharged into the municipal system and the equivalent attenuated volume for the 24 hour 1:1-year storm event. The fluctuation outflow is due to fluctuating water levels due to difference in inflow and outflow. Refer to **Appendix B** for the 1:1-year, 24-hour storm flow calculations.

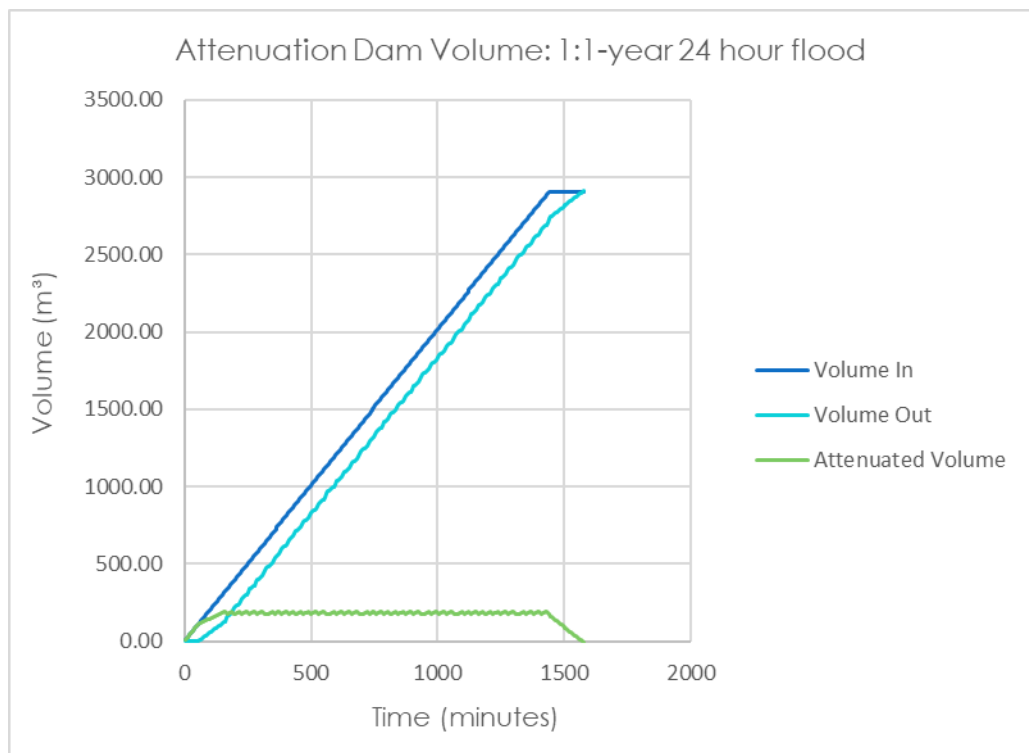


Figure 6: Attenuation Dam Volume: 1:1 year 24 hour storm event

The stormwater runoff inside the underground pipe network will pass through a stilling basin outlet structure before being discharged into the attenuation dams. **Figure 7** indicates an example of the proposed stilling basin into the attenuation dam. Refer to **Appendix F2** for the detailed drawing.



Figure 7: Proposed Stilling Basin Dam Inlet

Table 3 gives a summary of the total dam volume due to post development runoff, outflow rates, and the attenuated volume.

Design storm	Post-Dev Flow Rate	Pre-Dev Flow Rate	Total Outlet Capacity of Outlet Structure	Outlet Pipe Max Capacity	Max Outlet Capacity	Actual Attenuated Volume (Excl WQv)	Required Attenuation Volume (Based on Post - Pre)
	m ³ /s	m ³ /s	m ³ /s	m ³	m ³	m ³	m ³
1:2	0.49	0.34	0.3375	0.3089	0.3089	328.94	280.54
1:5	0.75	0.449	0.3932	0.3089	0.3089	785.32	533.82
1:10	0.94	0.534	0.4858	0.3089	0.3089	1133.16	728.10
1:20	1.13	0.619	0.5260	0.3089	0.3089	1481.00	922.37
1:50	1.42	0.743	0.5983	0.3089	0.3089	2007.73	1226.19
1:100	1.69	0.845	0.8482	0.3089	0.3089	2491.42	1526.59

Table 3: Attenuation dam design storm volumes

The maximum full storage capacity of the dam is 5087 m³. The proposed outlet structure (see **Appendix F3**), is designed to release the attenuated volume into the municipal network at pre-development, or as close as possible to pre-development flow rates with a standardised outlet size. The proposed outlet pipe from the dam to the municipal network will however govern flow rates, with a maximum flow capacity of 0.309 m³/s, which is less than pre-development flow rates.

The emergency overflow system is designed to accommodate storms larger than the 1:100-year storm. A grass block emergency overflow channel for extreme or abnormal flooding events is proposed on the

northwestern bank of the dam. **Figure 8** shows an example of the proposed overflow channel.



Figure 8: Proposed overflow channel

For full dam capacity and outlet sizing calculations, refer to **Appendix D**.

The developer aims to achieve LEED platinum accreditation. As a requirement, 20.1mm/m² of runoff should be attenuated. The attenuation facility has sufficient capacity to accommodate this volume of 2211m³.

3.5. STORMWATER QUALITY IMPROVEMENT

The CCT Management of Urban Stormwater Impacts Policy requires the improvement of the quality of runoff, through a combination of on-site and regional treatment facilities. The target is to reduce at least 80% of total suspended solids (TSS), and 45% of phosphorus content, for a volume equal to the 1:½-year 24-hour storm event.

The required water quality volume for the BELCON development is 1755.3 m³, calculated using the method as per the Georgia Stormwater Management Manual, as follow:

$$WQ_V = \frac{P \times R_v \times A}{1000}$$

With the following parameters:

- Precipitation depth (P): 19.528 mm
- Volumetric Runoff Coefficient (Rv): 0.8175
- Total Site Area (A): 110000 m²

The volumetric runoff coefficient, R_v , is calculated as follow:

$$R_v = 0.05 + (0.009 \times I)$$

With I equal to the percentage impermeable cover, in this case 85.23%, thus:

$$R_v = 0.05 + (0.009 \times 85.23) = 0.8175$$

Quality improvement targets will be reached through a combination of Best Management Practices along with Water Sensitive Urban Design initiatives, which includes rainwater harvesting, swales and a large attenuation dam which will promote infiltration and evaporation and the removal of suspended solids through settling and sedimentation control at inlet structures.

The attenuation dam in the northwestern corner of the site is the primary treatment facility for stormwater quality improvement, with the two swales also acting as treatment facilities for their respective contributing catchments, before releasing into the larger attenuation dam.

The swales are incorporated into the design as a best management practice. It will convey the runoff from its respective sub-catchments to the larger attenuation dam, which is designed with sufficient capacity for the whole site. Hence, the swales will provide additional storage to the more than sufficient primary attenuation facility, and will serve as pre-treatment facilities for their respective sub-catchments, before releasing into the main attenuation dam via grass block channel. A typical swale detail is included in **Appendix F**, with final levels and dimensions still to be formalised. The respective swale capacities are currently the following, as modelled with *Civil Designer*.

- Eastern Swale: 350³
- Western Swale: 64 m³

Refer to **Appendix G** for the proposed landscape plan for the BELCON Logistic Hub. The permeable, landscaped areas, promote infiltration and further improvement of runoff quality. The internal reuse of stormwater runoff for irrigation will also reduce the need for potable water. 70 000 litres, stored by means of tanks, will be used for irrigation purposes.

3.6. IMPACT OF MAJOR FLOWS

For the purpose of this design, major flows are classified as rainfall events with a return period larger than 1:10 -years, not included in the underground network capacity design.

The development is designed in such a manner that ponding will not disrupt the day-to-day activities within the development, or negatively impact the internal property or infrastructure. The topography and internal stormwater system of the proposed development site will ensure overland flow into the attenuation dam, which has capacity for the full volume generated from up to a 1:100-year peak storm event. Larger or abnormal rainfall event will be catered for by the north western overflow.

The current full dam capacity, excluding the additional swale storage capacity, is equal to 5086.91m³. The total volume generated by the peak 1:100-year storm event, for the total development site is 3047.38m³. The permanent water volume, is 2156.56m³ at full capacity. The combined permanent water volume and the peak 1:100-year generated runoff volume is 5203.94m³, not taking into account the outflow capacity. Based on a simplified, but conservative inflow vs outflow modelling of the attenuation dam, limited by the outflow pipe capacity, the maximum water level in the dam at any stage during the peak storm event will be 57.8 MSL, with the corresponding volume equal to 4820.42m³. See **Figure 9** for the graphical representation of the cumulative runoff volume generated, the total volume discharged into the municipal system and the equivalent attenuated volume for the peak 1:100 year storm event:

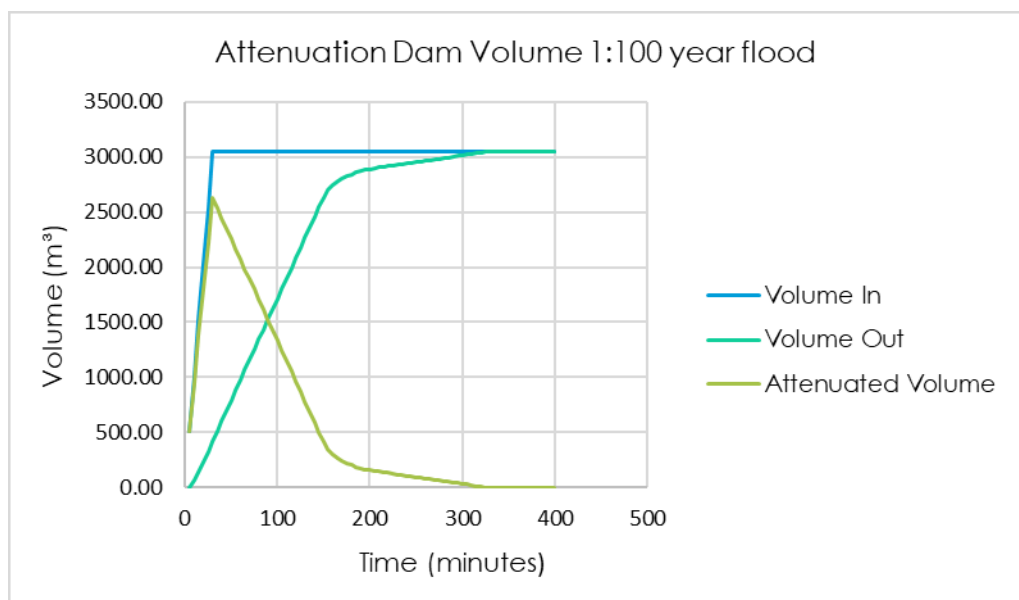


Figure 9: Attenuation Dam Volume: 1:100 year peak storm event

4. MAINTENANCE PROCEDURES

The internal stormwater system will be exposed to external elements and pollutants such as vehicle oils, litter and windblown sedimentation. This will significantly impact the operation and efficiency of the system. It is thus imperative that the stormwater infrastructure is maintained, checked, and cleaned at regular intervals to ensure optimal functioning of the system.

The completed development will be handed over to the facilities manager, who will be responsible for the maintenance and day-to-day operation of the development. This includes responsibility of the maintenance items specified in this section of the document.

It is imperative that maintenance personnel who carries out the routine inspections and maintenance procedures adhere to all Health and Safety standards, as per the Occupational Health and Safety Act (Act 85 of 1993).

4.1. ROUTINE CLEANING INSPECTIONS

Routine inspections and necessary cleaning should be undertaken on a regular basis by a competent, responsible person, such as a facilities manager or caretaker. It is recommended that routine inspections are done on a monthly basis, as well as after heavy rainfall events during the rainfall season. The inspection should include, but is not limited to:

- Checking that all gully-, grid- and kerb inlets are in place, clean and free of any blockages such as litter, leaves or sand build up.
- Manhole covers should be in place and free of any blockages.
- All stormwater pipes must be free of blockages.
- ACO-drains must be clean and clear of any litter build up or blockages.
- The level of sand in sand traps should be assessed and cleaned if necessary.
- Stilling basins at outlet structures should be checked for the level of sand and sedimentation build up.
- Open swales and the attenuation dam must be free of blockages to ensure free flow, and vegetation must be maintained.
- The structural integrity and water quality of rainwater harvesting tanks should be assessed, and all connections should be in place and free of leaks.

Any of the items which are deemed to not be satisfactory, must be tended to immediately.

4.2. ENGINEERING INSPECTIONS FOR MAINTENANCE

Engineering maintenance inspections should be undertaken on a yearly basis by a qualified engineer to assess the current condition of the stormwater infrastructure, to determine any repairs that may be required.

The water quality of the attenuation dams and runoff being discharged into the municipal system should be measured on a bi-yearly basis, by an independent laboratory.

The routine cleaning inspection checklist should also be assessed and updated as necessary.

The findings and recommendations of the maintenance inspection should be submitted to the client, who should immediately attend to any required remedial work, via a qualified contractor.

4.3. FINANCIAL ALLOWANCES FOR MAINTENANCE AND REPAIR WORKS

Although specific risk components can be identified at this stage of the development, it is very difficult to accurately determine the future vulnerability of the stormwater network. In addition to this, the future maintenance and replacement costs will largely depend on a number of building-industry and economic factors which is difficult to predict. In order to avoid large future expenditure on replacement and maintenance, we would recommend that the developer make allowance for a yearly stormwater maintenance budget of at least 5% of the total capital cost for the installation of the stormwater network.

4.4. SAFETY CONSIDERATIONS

The stormwater infrastructure must always be operated and maintained in accordance with all provisions of the Occupational Health and Safety Act (Act 85 of 1993).

In addition to the guidelines and requirements of the Occupational Health and Safety Act, it is also recommended that maintenance staff work in a safe manner. Appropriate personal protection equipment must be worn at all times during maintenance of the stormwater network, and any unsafe conditions should be reported immediately.

All manholes should be properly shut after maintenance procedures, to avoid tripping hazards. Open manholes should be cordoned off with appropriate danger tape.

5. CONCLUSION

CCT confirmed that they are in agreement with the KLS approach for stormwater management, *and detail design can proceed.*

This stormwater strategy plan is based on the design principles as set out in this document, as well as the conditions of approval set by the City of Cape Town. It is a holistic approach, which will significantly improve stormwater runoff generated by the new development in terms of quality and quantity, which will reduce strain on the existing municipal system.

We are therefore confident that the design and management plan complies with the City of Cape Town policies and requirements.



T Koegelenberg

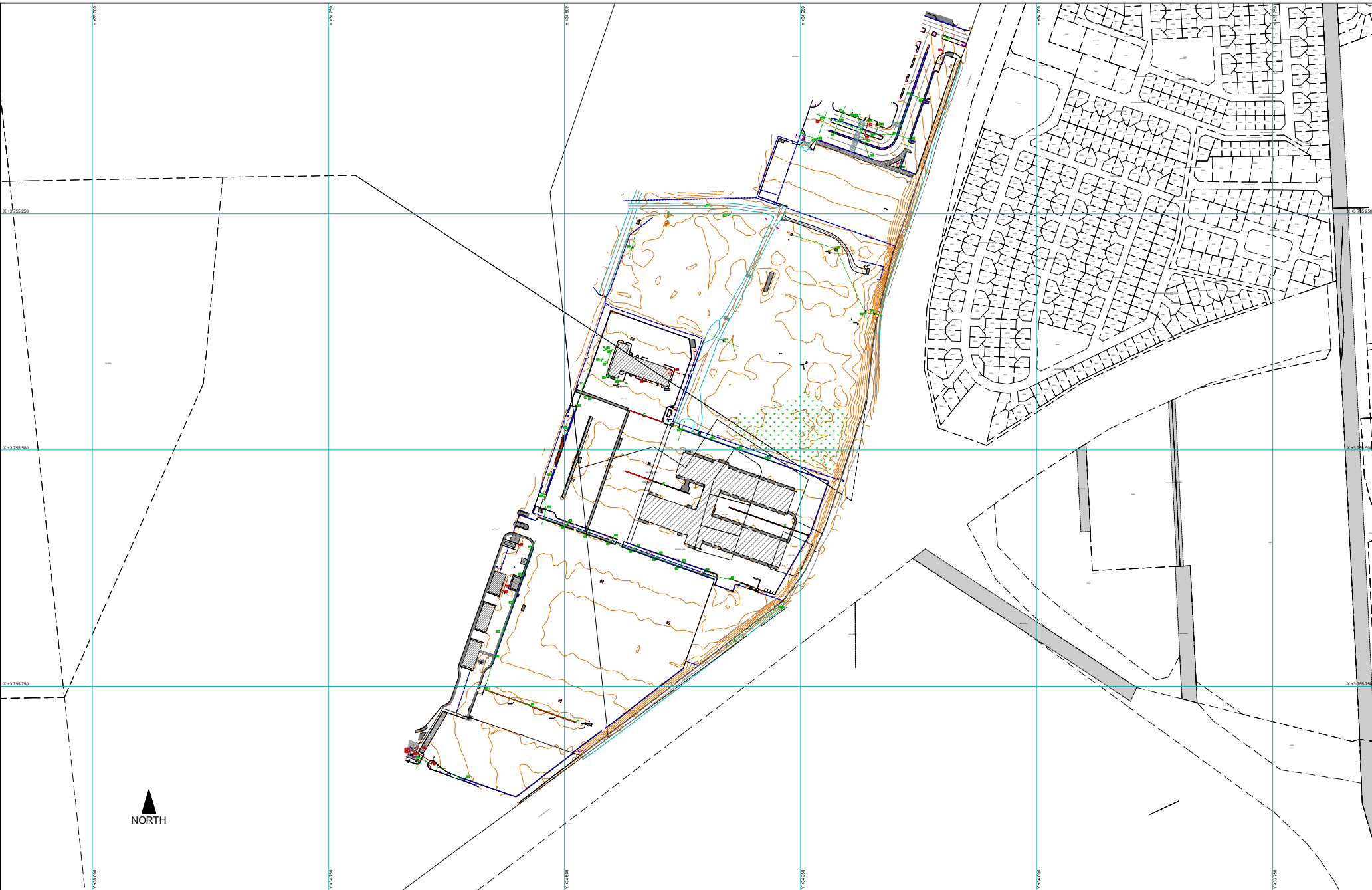


Mr. D.J. Kotze

Director

Appendix A

Topographical Survey



JOUBERT & BRINK SURVEYS
 TOPOGRAPHIC, ENGINEERING & CONSTRUCTION SURVEY SOLUTIONS

14 DELWYN CRESCENT
 STELLENBOSCH, 7500
 WESTERN CAPE
 SOUTH AFRICA

SAGC REGISTERED SURVEYORS

+27 (0)21 910-2004
 +27 (0)21 273-8266
 jbs@jmb.co.za
 CBK1995/0011306/23

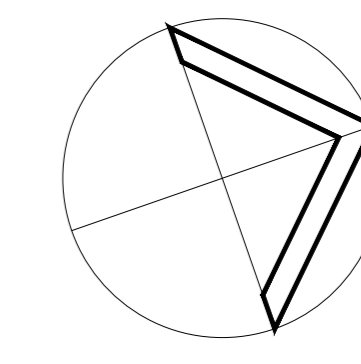
PLEASE NOTE

1. Only use our labelled control points. No other control points shall be used without consulting the surveyor.
2. All spot heights and positions are single measurement elevations and are therefore subject to error - any critical design points, especially services, should be discussed with the surveyor so that the appropriate survey methods can be used.
3. This survey includes only visible services which can be traced with hand tools and should be checked against local authority as-built plans. Services which are blocked or unable to be opened should be referred to the local authority. Invert levels deeper than 5 metres are approximate unless otherwise discussed before the survey commences. To save costs and time, a checked GIS overlay for information purposes are generally used, unless otherwise required.
4. Boundary measurements from Surveyor General diagrams are recommended for a time-consuming process. To save costs and time, a checked GIS overlay for information purposes are generally used, unless otherwise required.
5. This survey information is always sent as a signed data package which includes the drawing in DWG and PDF format, the raw data in ASCII format, a digital terrain model in XML, TDT, SDT and DSM format, Survey notes and descriptions. If there are any queries or requests regarding these files, please send us an email at jbs@jmb.co.za.

PROVISIONAL CONTROL BASED ON 284DC18, CHECKED ON 219DC18 AND 283DC18	
VERTICAL CONTROL BASED ON 284DC18, CHECKED ON 219DC18 AND 283DC18	
SURVEY LOCATOR WGS84	DESIGN BY H.V.D SANDT
REFERENCE EQUIPMENT WG19	DESIGNED BY C.STRYDOM
ALLOTMENT AREA BELLVILLE	DRAWN BY HARTY94
	ENGINEERED BY MAMSL
	DATE 03 OKT 2022
	PROJECT DESCRIPTION TOPOGRAPHICAL SURVEY OF BELCON MAERSK
	CLIENT MAERSK, BOTHA PRETORIUS
	DRAWING NAME
	SCALE 1:1250 (A0)
	DATE 9 of 9

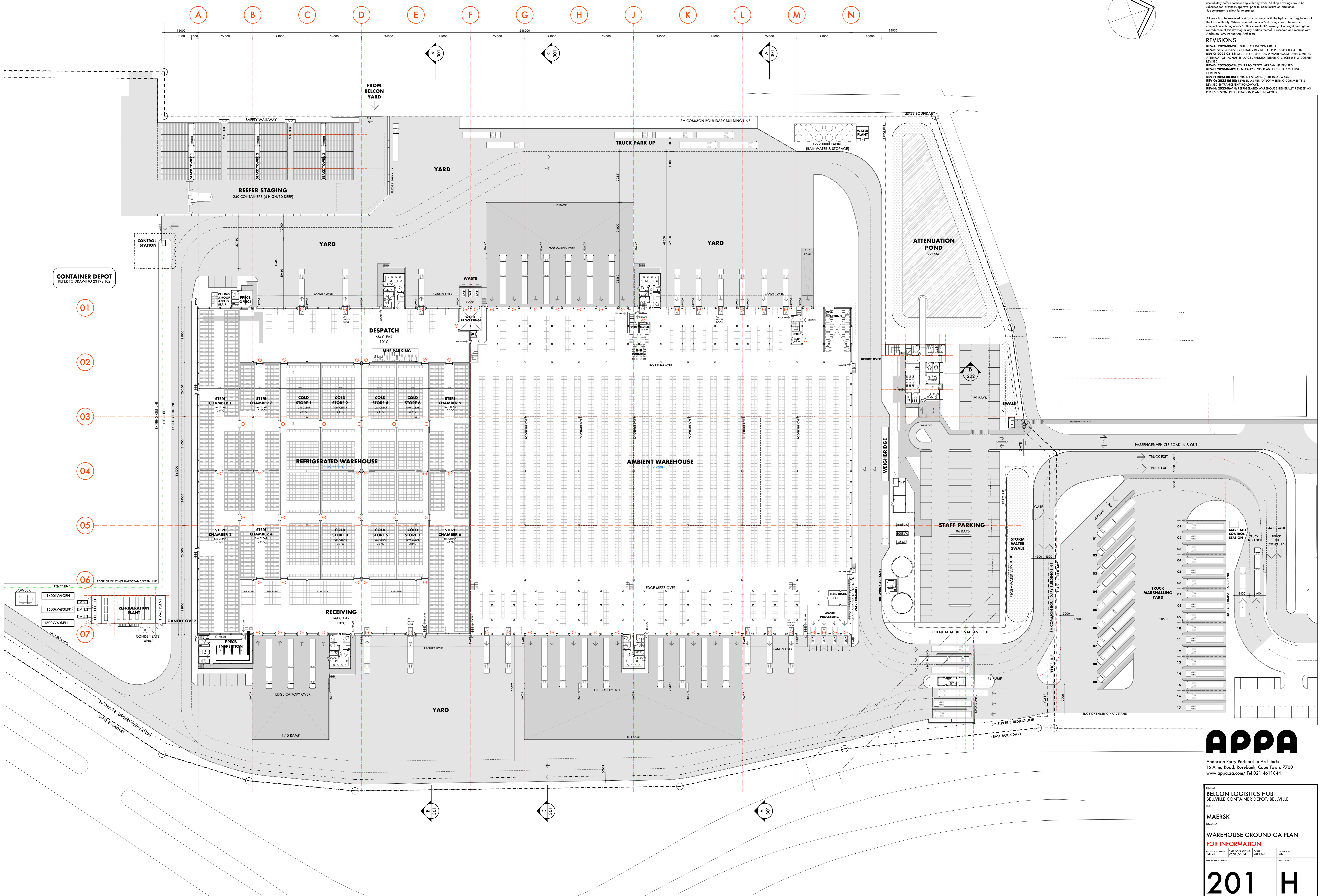
Appendix B

BELCON Site Plan



NOTES:
 All dimensions and levels are to be verified on site by the contractor and subcontractors, prior to the commencement of setting out, working drawings or construction. Drawings are not to be scaled. Figured dimensions only are to be used. Any discrepancies, errors and omissions are to be reported to the architect immediately, before commencing with any work. All shop drawings are to be submitted for architect approval prior to manufacture or installation. Subcontractor to allow for tolerances.

REVISIONS:
 REV-A: 2023-03-28: ISSUED FOR INFORMATION
 REV-B: 2023-05-09: GENERALLY REVISED AS PER ILS SPECIFICATION.
 REV-C: 2023-05-18: SECURITY TUNNELS IN WAREHOUSE LEVEL OMITTED. ATTENUATION PONDS ENLARGED/ADDED. TURNING CIRCLE @ RWY CORNER REVISED.
 REV-D: 2023-05-24: STAIRS TO OFFICE MEZZANINE REVISED.
 REV-E: 2023-06-05: GENERALLY REVISED AS PER "DITIG" MEETING COMMENTS.
 REV-F: 2023-06-05: REVISED ENTRANCE/EXIT ROADWAYS.
 REV-G: 2023-06-08: REVISED AS PER "DITIG" MEETING COMMENTS & REVISED ENTRANCE/EXIT ROADWAYS.
 REV-H: 2023-06-14: REFRIGERATED WAREHOUSE GENERALLY REVISED AS PER ILS DESIGN. REFRIGERATION PLANT ENLARGED.



CONTAINER DEPOT
 REFER TO DRAWING 23196.102

01
 02
 03
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APPA
 Anderson Perry Partnership Architects
 16 Alma Road, Rosebank, Cape Town, 7700
 www.appa.za.com / Tel 021 4611844

PROJECT
BELCON LOGISTICS HUB
 BELLVILLE CONTAINER DEPOT, BELLVILLE
 CLIENT
MAERSK
 DRAWING
WAREHOUSE GROUND GA PLAN
FOR INFORMATION
 PROJECT NUMBER: 23196 DATE OF THIS ISSUE: 25/03/2023 SCALE: A01:500 DRAWN BY: AS
 REVISION NUMBER: 23196 REVISION: AS

201 H

Appendix C – Departemental Reccommendations

C1. Water and Waste Directorate

C2. Road Infrastructure Management

C3. Environmental Management



MEMORANDUM

Date : 09 June 2023

To : Darrel Stevens and Ingrid Crous
(Development Management: Planning and Building Development Management)

Subject : **DAMS: LUMA – APPLICATION FOR FOR SUBDIVISION, CONSOLIDATION OF SUBDIVIDED SECTIONS, REZONING OF CONSOLIDATED ERF, CONSENT USE, PERMANENT REGULATION DEPARTURES, SDP APPROVAL AND REGISTRATION OF SERVITUDE ON ERVEN 20414, R/14867, 14881, R/14882, 14876, 14877, 14878 AND 14873 BELLVILLE.**

Our Ref : 2.3.2.7 Development Proposals (Elsieskraal River)

Applications with case number **1500100969_Erf 14881**) refers.

Herewith the Catchment Stormwater & River Management branch (this Office) recommendations for inclusion in the assessment of the aforementioned application applications.

CONTEXT

Nuplan Africa Town Planners have been appointed by Maersk Logistics & Services South Africa (Pty) LTD to lodge a land use application to the City of Cape Town on their behalf to enable the development of a logistics hub on a portion of land that they lease from Transnet at the existing Transnet Mashalling Yard in Bellville.

The application entails the following elements:

- Application for subdivision of 3 erven and consolidation of the subdivided portions and other erven.
- Rezoning of the consolidated portion to General Industrial Zone 1 and consent use for a container depot.
- Departure from the parking requirements and height restriction.
- Approval of the SDP.
- Registration of services servitudes and access servitudes.

The application consists of the following in terms of the City of Cape Town's Municipal Planning By-Law (2015):

1. In terms of Section 42(a) (rezoning of land, including rezoning to subdivisional area overlay zoning)
2. In terms of Section 42(b) (permanent departure)
3. In terms of Section 42(d) (Subdivision)
4. In terms of Section 42(i) (consent, approval or any other permission or requirement in terms of the development management scheme)

5. In terms of Section 42(u) (any other application which the City Manager may prescribe in terms of this By-Law)

COMMENTS

1. Taking into consideration the extent and nature of the proposed ultimate development, it is crucial that a sustainable stormwater management system be design, which is conducive to the Health & Safety of the general public and improves / protects the environment/ ecosystem.
2. As a precursor a stormwater strategy has been submitted as part of this application.
3. The submitted stormwater strategy of this precinct complies with the City of Cape Town's (the City) Stormwater Policies and By-law, noted here below:
 - 3.1 Management of Urban Stormwater Impacts Policy, approved by Council: 27 May 2009.
 - 3.2 Floodplain and River Corridor Management Policy, approved by Council: 27 May 2009.
 - 3.3 By-Law Relating to Stormwater Management, approved by Council: 30 August 2005.

Conditions

This Office supports this application, subject to the following conditions:

1. A Stormwater Management Plan (SWMP) must be submitted to this Office for approval prior to any building plan applications and/or submission.
2. The SWMP must comply with the City's Stormwater Policies and By-law, noted here below:
 - 2.1 Management of Urban Stormwater Impacts Policy, approved by Council: 27 May 2009.
 - 2.2 Floodplain and River Corridor Management Policy, approved by Council: 27 May 2009.
 - 2.3 By-Law Relating to Stormwater Management, approved by Council: 30 August 2005.
3. The appointed Design Engineer (Engineering Consultancy) and/or applicant is encouraged to consult with this office prior to the compilation of the SWMP to discuss its principles.
4. The Developer is encouraged to implement stormwater Best Management Practices (BMP's) along with other Water Sensitive Urban Design (WSUD) initiatives such as reduction of potable water use through rainwater harvesting, bio-retention areas, swales, rain gardens, etc.
5. Indicate stormwater servitudes for under/above ground services, e.g. overland escape routes etc. Obtain written approval from affected owners, where the route of a proposed civil service crosses private properties and register servitudes accordingly. Maintenance responsibility of servitudes must be included into the servitude conditions. All cost incurred is for the developer's account.
6. It should be noted, that this Office accepts all SWMP in good faith. Furthermore, this Office trusts that the developer/applicant/his professional team will incorporate into the SWMP, the City's internal departments, as well as Organs of State's conditions (i.e. authorisations and/or water use licenses, etc.). Failure to do so will result in unnecessary delays, and all costs associated with delays will be for the Developer's account.
7. Floodline maps/data provided by this Office were prepared for the purpose of assessing the degree of flood hazard and risk to assist in the identification and development of measures for managing the flood risk. They may, however, also be of use to the public and other parties as indicative floodlines of flood-prone areas for a range of purposes, including raising awareness of

WATER & SANITATION HEAD OFFICE

8 VOORTREKKER ROAD, CNR OF MIKE PIENAAR BOULEVARD, BELLVILLE 7535 PRIVATE BAG X98, BELLVILLE, 7535
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flood risk, preparedness and response planning for flood events, assisting in planning and development decisions, etc.

8. The City's High Level Master Plan Floodlines are coarse; therefore, floodlines compiled prior to 2013 must be verified/vetted by the Developer's professional team to establish flood risk, etc. All costs incurred is for the Developer's account. Furthermore, this Office reserves the right to change the content and/or presentation of any of the information provided on these floodline maps at its sole discretion, including these notes and disclaimer.
9. All open spaces must be must be zoned as private open space and to be maintained by the overarching home owners' association and not public open space.

Please note that these comments are based on the information that has been received to date. Should any new information be provided to this office, then this office reserves the right to review the recommendations as deemed appropriate.

Yours faithfully,

Motlatsi Christian
Nkhoesa
p.p.
Johann Terblanche

Digitally signed by
 Motlatsi Christian Nkhoesa
 Date: 2023.06.09 11:13:25
 +02'00'

Head: Catchment Planning: Region 4 (Northern Districts)

CC:

Sandra Hustwick	: Spatial Planning and Environment: Environment & Heritage Management (Northern region)
Louise du Toit	: Transport: Road Infrastructure Management (District 3)
Daneel du Toit	: Transport: Road Infrastructure Management (District 3)
Motlatsi Nkhoesa	: Water & Waste Services: Assistant Catchment Planner: Region 4 (Northern Districts)
Kloey Bam	: Water & Waste Services: Assistant Catchment Planner: Region 4 (Northern Districts)
Willem Burger	: Water & Waste Services: Assistant Catchment Planner: Region 4 (Northern Districts)
Phila Nkosinkulu	: Community Services and Health: Recreation and Parks



MEMORANDUM

CASE ID **1500100969**
 DATE 30 June 2023
 To Darrel Stevens (*Development Management*)
 APPLICATION **REZONING, SUBDIVISION, CONSOLIDATION, CONSENT USE, PERMANENT DEPARTURE ERVEN RE-20414, RE-14882, RE-14867, 14881, 14873, 14876, 14877 AND 14878, BELLVILLE (TRANSNET MARSHALLING YARD), BELLVILLE**

1. NATURE OF APPLICATION:

- 1.1 **Subdivision** of erven 20414, 14881 and RE-14882 into 2 portions each.
- 1.2 **Rezoning** of portion 1 of erven 20414 and RE-14882 from TR1 to GI 1 purposes
- 1.3 **Consolidation** of RE-14867, 14876, 14877, 14878, 14873, portion 1 of 20414, portion 1 of 14882 and portion 1 of 14881 with a right of way servitude.
- 1.4 **Consent use** to allow for a Container Depot to be operated from the newly consolidated property
- 1.5 **Parking departure** from the prescribed 408 bays to 274 bays and
- 1.6 Departure to increase the permissible height of 18m to 24m.

2. COMMENTS:

- 2.1 Our GIS records indicates an existing stormwater culvert traversing the site.
- 2.2 Existing rights: Six of the eight land portions have existing General Industrial 1 rights. The total combined area of these land portions is approximately 75,000m² which translates to an existing right of 112,800m² floor area. Therefore no Development Charges will be payable.
- 2.3 The access, transport impact, circulation and parking must be to the satisfaction of the *Urban Mobility: TIA & Development Control Branch*.
- 2.4 The application is to consolidate and rezone the full Lease Area to Industrial Zone 1 to permit:
 - (a) 40,782 m² GLA warehousing / cold storage
 - (b) 40,780 m² of Container Yard (as a consent use)

USE	GROSS LEASABLE AREA (GLA)
Warehouse (Ambient)	20646m ²
Warehouse (Mezzanine)	4299m ²
Warehouse (Cold Storage)	12774m ²
Pilot offices & Staff Entrance	951m ²
Offices (Mezzanine)	2 058m ²
Gatehouse	54
Total	40782m²

2.5 Servitudes:

- (a) The proposed R.O.W. servitudes (to provide access to the subject site from the north and the south) must be illustrated and dimensioned on the SDP and shown on the Consolidation Plan.
- (b) An existing stormwater culvert traverses the site – according to the motivation report allowance has been made for a 12m wide servitude, however the SDP indicates a 6m wide servitude. Please clarify. Also, indicate the servitude width on the Consolidation Plan.

- 2.6 During a meeting with the consulting engineers on the 23rd of February 2023 the City requested that they include a cut-off drain along the railway lines - tying it into an existing earth drain. GIBB Consulting Engineers analysed the stormwater run-off for CCT and this led to the size of the cut-off drain to be 0.5m deep, 0.5m wide with side slopes of 1:3 at the start. The cut of drain increases to 1.2m deep, 1.2m wide with side slopes of 1:3 at the end. We recommended that KLS incorporate the cut-off drain into the works as this will be a condition with the SDP approval.
- 2.7 During abovementioned meeting a possible trapped low point in Robert Sobukwe Road was discussed. The engineers confirmed that they have survey data and will confirm if an overland escape route across the site will have to be accommodated. This was not mentioned in the engineering services report or the stormwater strategy plan – please confirm as this would require a servitude.

3. RECOMMENDATIONS:

This Department **reserves final comment** on the application until the abovementioned comments have been addressed.

Yours sincerely,

Louise du Toit

Roads Infrastructure Management
Urban Mobility Directorate



INTERNAL MEMORANDUM

T: 021 400 6534

[e: Ronelle.Clarke@capetown.gov.za](mailto:Ronelle.Clarke@capetown.gov.za)

To : Darrel Stevens (Land Use Management)

From : Ronelle Clarke (Environment & Heritage Management Branch E&HM)

Subject : Erven 20414, R/14867, 14881, R/14882, 14876, 14877, 14878 & 14873 14881, Bellville, Case 1500100969.

Application for subdivision, consolidation of subdivided sections, rezoning of consolidated erf, consent use, permanent regulation departures, SDP approval and registration of servitudes

Date : 7 June 2023

Comment:-

- 1 The application as described above is to allow for the development of cold storage, warehousing and container depot facilities on a portion of land at the existing Transnet Marshalling Yard in Bellville.
- 2 A pre-consultation meeting and follow up was undertaken between the applicant and Environmental Management.



Figure 1: The site in question

- 3 E&HM supports the proposed application. There are no environmental constraints / issues that require further assessment or checks.
- 4 The Landscape Plan (LP) by CNDV Drwg No 2976 - 03 - LP - REV 01 dated 5 April 2023 (attached here for reference purposes only), is accepted BUT subject to the following amendments:

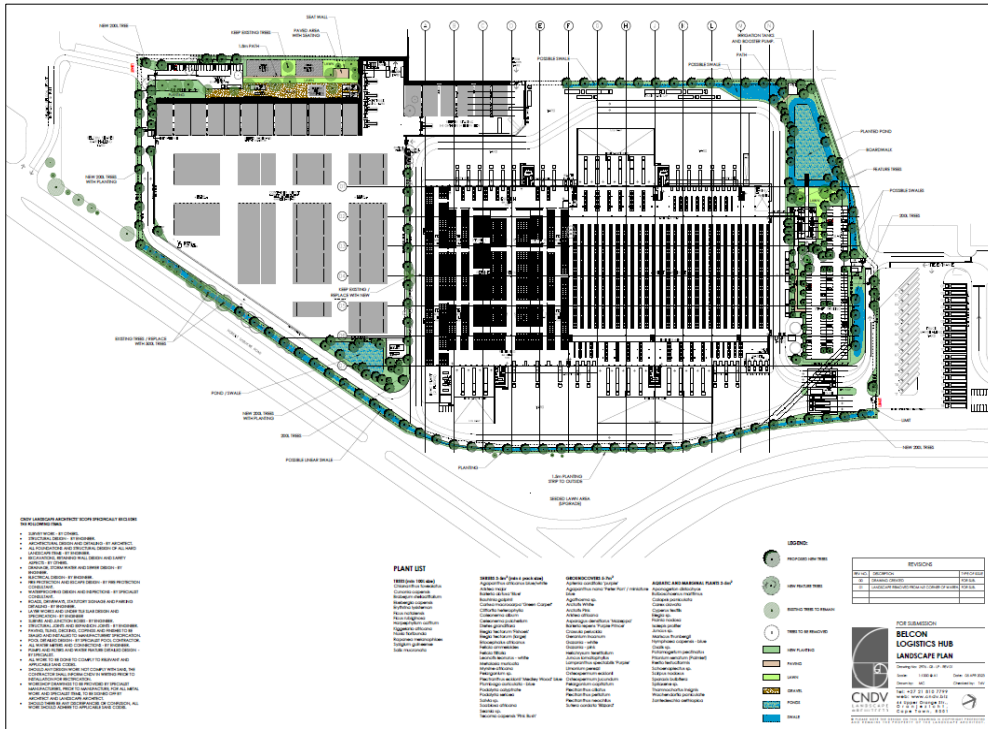


Figure 2: Landscape Plan by CNDV Drwg No 2976 - 03 - LP - REV 01 dated 5 April 2023

- 4.1 The interface between Robert Sobukwe and the proposed development is important and tree planting for screening and visual amenity is required. The proposed 200L trees are accepted. However, the Plan must be amended to indicate that the row of existing *Casuarina* trees along Robert Sobukwe (see Figure 3 below) are to be retained and that new planting is to take place alongside and continuing from these existing trees.

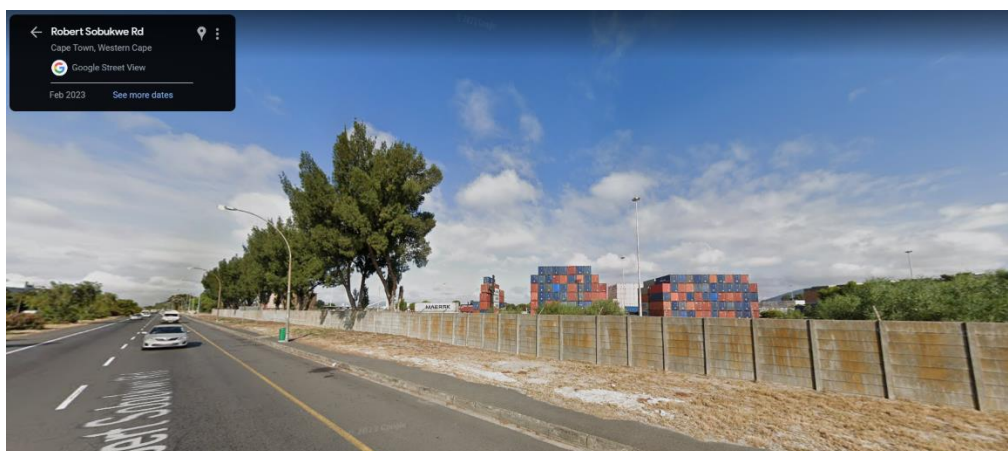


Figure 3: Existing row of mature trees on Robert Sobukwe Road

- 4.2 The proposed tree species and diversity proposed is welcomed. However, remove the proposed tree species *Kiggelaria africana* (Wild Peach) from the suggested species to plant as this has been identified as a reproductive host species for the Polyphagus Shothole Borer, an alien invasive beetle that is causing the demise of trees in Cape Town and surrounds.
- 4.3 Show and provide on the LP details of irrigation to all new tree planting (to be a fully automated irrigation system). Indicate options for alternative sources of water.
- 4.4 Note on the LP maintenance of planting. A watering program is needed that is the responsibility of the developer for a minimum of 2 years after planting to ensure that trees can establish themselves.
- 4.5 Runoff from all hard surfaces and internal roads should remain on site for irrigation use / flow into adjacent vegetated areas. Surface treatment should be laid to fall towards the trees and planted areas to allow for passive irrigation. The proposed linear swales to allow for Sustainable Urban Drainage Systems (SUDS) is supported.
- 4.6 Show boundary treatment (is the existing vibracrete wall on Robert Sobukwe to remain?)
- 5 Note to LUM:
 - 5.1 Attach as a condition of approval a time frame for implementation of all landscaping. Environment to be notified at such time so that a site visit can be arranged to confirm such implementation.
 - 5.2 Conditions contained in Heritage Western Cape's response to the Notification of Intent to Develop dd 6 April 2023 must be carried through / referred to at approval.

RS Clarke

Ronelle Clarke
Environmental Professional
ENVIRONMENTAL AND HERITAGE MANAGEMENT: TYGERBERG

Appendix D


D1. Pre-Development Runoff Calculations

D2. Post-Development Runoff Calculations

D3. Runoff Calculation Summary

D4. Outlet and Overflow Sizing

D5. Flow modelling & graphs


	FLOOD RUN-OFF CALCULATIONS	PROJECT:	BELCON	
	Project no:	22093	Page No	2
	Catchment Name:	BELCON	Rev:	B
	Calculation Method:	SANRAL AND RATIONAL METHOD (ALTERNATIVE 3)		
	Calculated by:	Neil Wolter		
	Date:	2023-02-20		
	Checked By:	Neil Wolter	Signed:	
Date:	2023-02-20			

Flood run-off: Rational method (Alternative 1)								
Return Period (years), T			2	5	10	20	50	100
Point rainfall (mm), P _T			6.50	9.00	11.00	13.00	18.00	25.00
Point intensity (mm/hour), P _{IT} (= P _T /T _c)			13.90	19.25	23.52	27.80	38.49	53.46
Area reduction factor (%), ARF _T			100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I _T			13.90	19.25	23.52	27.80	38.49	53.46
Peak flow (m ³ /s)	$Q_T = \frac{C_T I_T A}{3.6}$	$Q_T = \frac{C_T I_T A}{3.6}$	0.18	0.25	0.30	0.36	0.50	0.69

Flood run-off: Rational method (Alternative 2)								
Return Period (years), T			2	5	10	20	50	100
Rainfall duration (minutes) t			28.06	28.06	28.06	28.06	28.06	28.06
Point rainfall (mm), P _T <i>PtT = 1.13(0.41+0.64LnT)(-0.11+0.27LnT)(0.79M^{0.69}R^{0.20})</i>			9.45	15.95	20.86	25.77	32.26	37.17
Point intensity (mm/hour), P _{IT} (= P _T /T _c)			20.21	34.10	44.60	55.11	68.99	79.50
Area reduction factor (%), ARF _T			100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I _T			20.21	34.10	44.60	55.11	68.99	79.50
Peak flow (m ³ /s)	$Q_T = \frac{C_T I_T A}{3.6}$	$Q_T = \frac{C_T I_T A}{3.6}$	0.26	0.44	0.57	0.71	0.89	1.02

Flood run-off: Rational method (Alternative 3)								
Return Period (years), T			2	5	10	20	50	100
Point rainfall (mm), P _T			12.20	16.30	19.40	22.50	27.00	30.70
Point intensity (mm/hour), P _{IT} (= P _T /T _c)			26.09	34.86	41.49	48.12	57.74	65.65
Area reduction factor (%), ARF _T			100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I _T			26.09	34.86	41.49	48.12	57.74	65.65
Peak flow (m ³ /s)	$Q_T = \frac{C_T I_T A}{3.6}$	$Q_T = \frac{C_T I_T A}{3.6}$	0.34	0.45	0.53	0.62	0.74	0.84

Flood run-off: SDF									
Return Period (years), T			2	5	10	20	50	100	200
Basin			17.0						
Mean Annual Precipitation (MAP)			500.0						
Mean 24 hour maximum rainfall (1:2) - M			45.0						
Mean number of thunder days per annum - R			1.0						
Rainfall duration (minutes) t			28.06	28.06	28.06	28.06	28.06	28.06	28.06
Point rainfall (mm), P _T <i>PtT = 1.13(0.41+0.64LnT)(-0.11+0.27LnT)(0.79M^{0.69}R^{0.20})</i>			8.33	14.05	18.37	22.70	28.42	32.75	37.07
Area reduction factor (%), ARF _T			100%	100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I _T = P _{IT} x ARF / T _c			17.81	30.04	39.29	48.54	60.78	70.03	79.28
Run-Off Coefficients									
Calibration Factors	C ₂ (2-year return period) (%)	40	C ₁₀₀ (2-year return period) (%)			80			
Return Period (years), T			2	5	10	20	50	100	200
Return Period Y _T Factors			0.00	0.84	1.28	1.64	2.05	2.33	2.580
Run-off coefficient (C _T),			0.40	0.54	0.62	0.68	0.75	0.80	0.84
Peak flow (m ³ /s)	$Q_T = \frac{C_T I_T A}{3.6}$	$Q_T = \frac{C_T I_T A}{3.6}$	0.22	0.50	0.75	1.01	1.40	1.72	2.05

FLOOD RUN-OFF CALCULATIONS		PROJECT:	BELCON		
	Project no:	22093		Page No	2
	Catchment Name:	BELCON		Rev:	B
	Calculation Method:	SANRAL AND RATIONAL METHOD (ALTERNATIVE 3)			
	Calculated by:	Neil Wolter			
	Date:	2023-02-20			
	Checked By:	H Kotze		Signed:	
Date:	2023-02-20				

Flood run-off: Rational method (Alternative 1)							
Return Period (years), T		2	5	10	20	50	100
Point rainfall (mm), P_T		6.50	9.00	11.00	13.00	18.00	25.00
Point intensity (mm/hour), P_{IT} (= P_T/T_C)		13.90	19.25	23.52	27.80	38.49	53.46
Area reduction factor (%), ARF_T		100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I_T		13.90	19.25	23.52	27.80	38.49	53.46
Peak flow (m^3/s)	$Q_T = \frac{C_T I_T A}{3.6}$	0.35	0.49	0.59	0.70	0.97	1.35

Flood run-off: Rational method (Alternative 2)							
Return Period (years), T		2	5	10	20	50	100
Rainfall duration (minutes) t		28.06	28.06	28.06	28.06	28.06	28.06
Point rainfall (mm), P_T		9.45	15.95	20.86	25.77	32.26	37.17
Point intensity (mm/hour), P_{IT} (= P_T/T_C)		20.21	34.10	44.60	55.11	68.99	79.50
Area reduction factor (%), ARF_T		100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I_T		20.21	34.10	44.60	55.11	68.99	79.50
Peak flow (m^3/s)	$Q_T = \frac{C_T I_T A}{3.6}$	0.51	0.86	1.12	1.39	1.74	2.00

Flood run-off: Rational method (Alternative 3)							
Return Period (years), T		2	5	10	20	50	100
Point rainfall (mm), P_T		12.20	16.30	19.40	22.50	27.00	30.70
Point intensity (mm/hour), P_{IT} (= P_T/T_C)		26.09	34.86	41.49	48.12	57.74	65.65
Area reduction factor (%), ARF_T		100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), I_T		26.09	34.86	41.49	48.12	57.74	65.65
Peak flow (m^3/s)	$Q_T = \frac{C_T I_T A}{3.6}$	0.66	0.88	1.05	1.21	1.46	1.65

Flood run-off: SDF								
Return Period (years), T		2	5	10	20	50	100	200
Basin					17.0			
Mean Annual Precipitation (MAP)					500.0			
Mean 24 hour maximum rainfall (1:2) - M					45.0			
Mean number of thunder days per annum - R					1.0			
Rainfall duration (minutes) t		28.06	28.06	28.06	28.06	28.06	28.06	28.06
Point rainfall (mm), P_T								
$P_T T = 1.13(0.41+0.64 \ln T)(-0.11+0.27 \ln T)(0.79 M^{0.69} R^{0.20})$		8.33	14.05	18.37	22.70	28.42	32.75	37.07
Area reduction factor (%), ARF_T		100%	100%	100%	100%	100%	100%	100%
Average intensity (mm/hour), $I_T = P_{IT} \times ARF_T / T_C$		17.81	30.04	39.29	48.54	60.78	70.03	79.28
Run-Off Coefficients								
Calibration Factors	C_2 (2-year return period) (%)	40	C_{100} (2-year return period) (%)		80			
Return Period (years), T		2	5	10	20	50	100	200
Return Period Y_T Factors		0.00	0.84	1.28	1.64	2.05	2.33	2.580
Run-off coefficient (C_T),		0.40	0.54	0.62	0.68	0.75	0.80	0.84
Peak flow (m^3/s)	$Q_T = \frac{C_T I_T A}{3.6}$	0.22	0.50	0.74	1.01	1.40	1.71	2.04

Flow (Q - m ³ /s)	Summary of Flow: Pre-Development						Summary of Flow: Post-Development					
	Alternative 1	Alternative 2	Alternative 3	SDF	KLS Design		Alternative 1	Alternative 2	Alternative 3	SDF	KLS Design	
Q ₂	0.18	0.26	0.34	0.22	0.25	Q ₂	0.35	0.51	0.66	0.22	0.43	
Q ₅	0.25	0.44	0.45	0.50	0.41	Q ₅	0.49	0.86	0.88	0.50	0.68	
Q ₁₀	0.30	0.57	0.53	0.75	0.54	Q ₁₀	0.59	1.12	1.05	0.74	0.88	
Q ₂₀	0.36	0.71	0.62	1.01	0.67	Q ₂₀	0.70	1.39	1.21	1.01	1.08	
Q ₅₀	0.50	0.89	0.74	1.40	0.88	Q ₅₀	0.97	1.74	1.46	1.40	1.39	
Q ₁₀₀	0.69	1.02	0.84	1.72	1.07	Q ₁₀₀	1.35	2.00	1.65	1.71	1.68	

Intensities (mm/h)	Summary of Intensities: Pre-Development						Summary of Intensities: Post-Development					
	Alternative 1	Alternative 2	Alternative 3	SDF	KLS Design		Alternative 1	Alternative 2	Alternative 3	SDF	KLS Design	
I ₂	13.90	20.21	26.09	17.81	19.50	I ₂	13.90	20.21	26.09	17.81	19.50	
I ₅	19.25	34.10	34.86	30.04	29.56	I ₅	19.25	34.10	34.86	30.04	29.56	
I ₁₀	23.52	44.60	41.49	39.29	37.23	I ₁₀	23.52	44.60	41.49	39.29	37.23	
I ₂₀	27.80	55.11	48.12	48.54	44.89	I ₂₀	27.80	55.11	48.12	48.54	44.89	
I ₅₀	38.49	68.99	57.74	60.78	56.50	I ₅₀	38.49	68.99	57.74	60.78	56.50	
I ₁₀₀	53.46	79.50	65.65	70.03	67.16	I ₁₀₀	53.46	79.50	65.65	70.03	67.16	

CoCT Design Rainfall Grid Data for 24 hour storm event		
1440 minutes		
RI	Depth	Intensity
1:	mm	mm/h
0.5	19.528	0.814
1	32.022	1.334
2	44.516	1.855
5	61.032	2.543
10	73.526	3.064
20	86.020	3.584
50	102.536	4.272
100	115.030	4.793
200	127.524	5.314

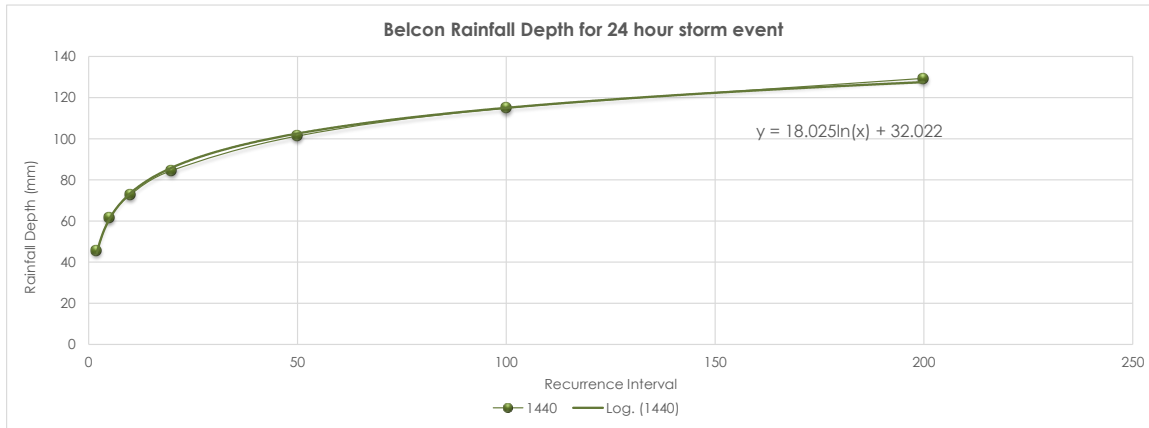
SANRAL Drainage Manual		
Pre-Dev Run-Off Coeff	C _{pre}	0.420
Post-Dev Run-Off Coeff	C _{post}	0.825

GSMM		
Volumetric Runoff Coefficient	R _v	0.8171
% Impervious Cover (post)		85%

Water Quality Improvement Requirement		Volume (m ³)
SUDS	1:½-year 24 hour storm	1755.30
Attenuation Requirements		Volume (m ³)
SUDS	1:1-year, 24 hour storm	2878.33
LEED	20.1mm/m ²	2211.00

**Area of 110000m²

**LEED Requirements met, includes PWL, Swale Capacity, and Rainwater harvesting tanks



Rainfall Data - Belcon Logistics Hub								
Reference Coordinates		33°55' ; 18°37'						
MAP (mm)		506						
Return Period	0.5	1	2	5	10	20	50	100
1-day Precipitation depth (mm)			39.3	52.8	62.6	72.7	87.1	98.8
440-min precipitation depth (mm)	19.5	32.0	44.5	61.0	73.5	86.0	102.5	115.0
30-min precipitation depth (mm)			12.2	16.3	19.4	22.5	27.0	30.7

ATTENUATION FACILITY OUTLET SIZING CALCULATIONS

	Dam	Swale West	Swale East	Total	
Contributing Area	89847	2250	17503	110000	m ²
Recurrence Interval	Post Development Flow				
2	0.402	0.010	0.080	0.49	m ³ /s
5	0.609	0.015	0.121	0.75	
10	0.766	0.019	0.153	0.94	
20	0.924	0.023	0.184	1.13	
50	1.163	0.029	0.232	1.42	
100	1.383	0.035	0.276	1.69	
	Total peak storm volume				
2	722.775	18.100	144.021	884.896	m ³
5	1095.542	27.435	218.299	1341.276	
10	1379.655	34.550	274.911	1689.117	
20	1663.769	41.665	331.524	2036.958	
50	2093.997	52.439	417.252	2563.688	
100	2489.069	62.333	495.974	3047.375	

Attenuation Dam Outlet Structures

Treatment Volume m³ 1755.30 m³

Assumptions:

- *Saturated Soil
- *Well pond, with P.W.L = Water Quality(Treatment) Volume
- *Sized to accommodate full catchment runoff contribution
- *Rectangular outlets
- *Sharp Inlets

Full Water Quality Volume: 2156.56 m³ (increased to ensure 1.5 m depth to prevent tree growth)
 Full Water Quality Level: 56.60 MSL

Design storm	Total Post - Development Volume	Max Water Level (Adjusted for outflow where needed)	Max Water Height Above Inlet	Post Development Flow Rate	Pre-development Flow Rate	Outflow Slots	Outlet Overflow	Total Outlet Capacity of Outlet Structure	Outlet Pipe Max Capacity	Max Outlet Capacity	Actual Attenuated Volume (Excl WQV)	Required Attenuated Volume (Based on Post - Pre)
1:2	m ³	MSL	m	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³	m ³
1:2	3041.46	57.00	0.40	0.49	0.34	0.14876	0.3375	0.3375	0.3089	0.3089	328.94	280.54
1:5	3497.84	57.10	0.50	0.75	0.449	0.19660	0.3932	0.3932	0.3089	0.3089	765.32	533.82
1:10	3845.68	57.30	0.70	0.94	0.534	0.24288	0.4858	0.4858	0.3089	0.3089	1133.16	728.10
1:20	4193.52	57.40	0.80	1.13	0.619	0.29299	0.5260	0.5260	0.3089	0.3089	1481.00	922.37
1:50	4720.25	57.60	1.00	1.42	0.743	0.29917	0.5983	0.5983	0.3089	0.3089	2007.73	1226.19
1:100	5203.94	57.8	1.20	1.69	0.845	0.33143	0.6629	0.6629	0.3089	0.3089	2491.42	1526.59

Alternative 3

Outlet Structure Sizing

Rectangular Inlet 1	
Inlet Level	56.6 MSL
Width (B)	0.6 m
Inlet Height (D)	0.2 m

Outlet Overflow	
Level	57.7 MSL
Width (B)	0.7 m
Water Height (D)	0.10 m
Bars Ø (m) x 7	0.01 m

**1:100 storm

Outlet Control: Culverts

$$Q = \frac{2}{3} \times C_d B H \sqrt{\frac{2}{3} g H}$$

For H/D <= 1.2
With C = 0.9

$$Q = C_d B D \sqrt{2g(H - C_d D)}$$

For H/D > 1.2
With C = 0.6

Orifice Control

$$Q = C_d \times F \times A \times \sqrt{2gH}$$

With C = 0.6
F = BLOCKAGE FACTOR OF 0.5

Emergency Overflow

Channel Sizing:	Manning
Flow Capacity (m ³ /s)	0.57
A (m ²)	0.485
P (m)	5.33
n	0.03
S (m/m)	0.03
*Trapezoidal Channel	
Side Slopes 1:	1.5
Top Width	5
Bottom	4.7
Height	0.1
Level (MSL)	57.8

**Grass Blocks

57.9

Outlet-Municipal connection pipe

Pipe Capacity:	Manning
Max Flow (m ³ /s)	0.309
A (m ²)	0.1590
P (m)	1.4137
n	0.012
S (m/m)	0.01
*Concrete Pipe	
Water Height	0.45
Design Level	0
Pipe Radius	0.225
Design Angle	0

*Concrete

*Approximate 11E IN @ 55.802 IN

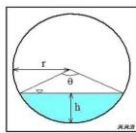
Input

*Radians

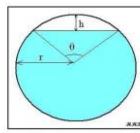
Swale conveyance/Overflow

Channel Sizing	Manning
Flow Capacity	0.24
A (m ²)	0.3
P (m)	3.03
n	0.03
S (m/m)	0.0125
*V-Channel	
Side Slopes 1:	7.5
Top Width	3
Height	0.2
Level (MSL)	58.1

**Grass Blocks

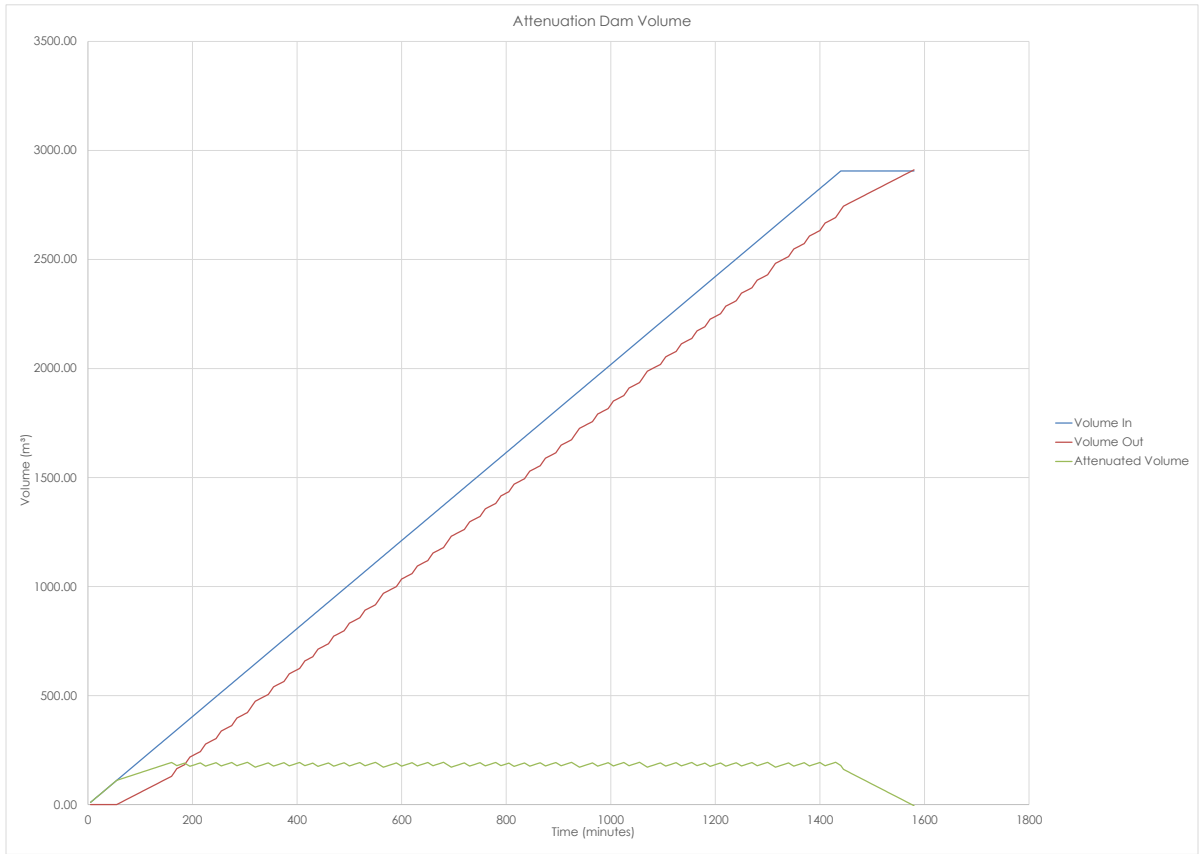


Partially Full Pipe Flow Parameters

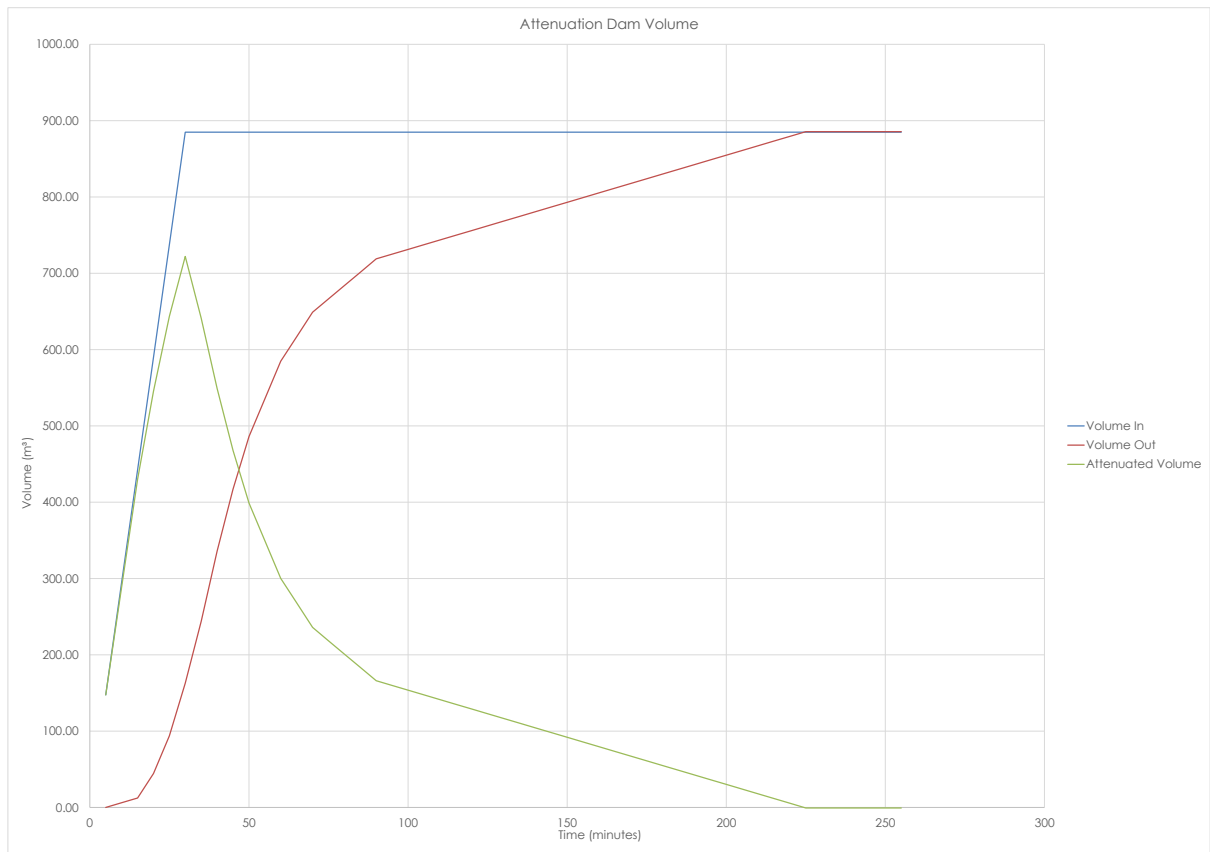


Partially Full Pipe Flow Parameters

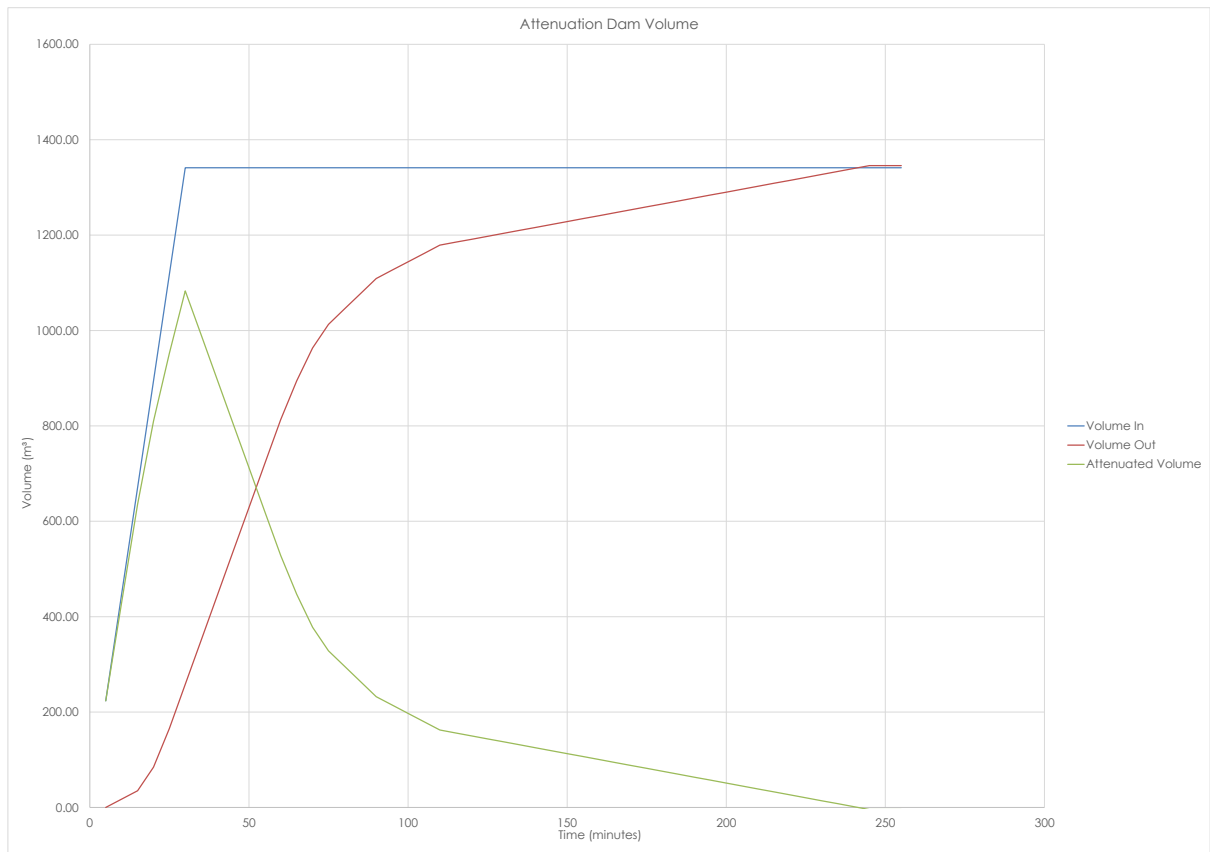
770	10.90	1553.90	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1368.709504	2341.75	185.19	56.65
775	10.90	1563.99	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1374.885127	2345.47	189.11	56.70
780	10.90	1574.08	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1381.060750	2349.18	193.03	56.70
785	10.90	1584.17	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1387.236373	2352.90	196.95	56.65
790	10.90	1594.26	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1393.411996	2356.61	200.87	56.65
795	10.90	1604.35	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1400.000000	2360.32	204.79	56.65
800	10.90	1614.44	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1406.588004	2364.03	208.71	56.65
805	10.90	1624.53	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1413.176008	2367.74	212.63	56.65
810	10.90	1634.62	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1419.764012	2371.45	216.55	56.65
815	10.90	1644.71	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1426.352016	2375.16	220.47	56.65
820	10.90	1654.80	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1432.940020	2378.87	224.39	56.65
825	10.90	1664.89	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1439.528024	2382.58	228.31	56.65
830	10.90	1674.98	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1446.116028	2386.29	232.23	56.65
835	10.90	1685.07	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1452.704032	2390.00	236.15	56.65
840	10.90	1695.16	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1459.292036	2393.71	240.07	56.65
845	10.90	1705.25	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1465.880040	2397.42	243.99	56.65
850	10.90	1715.34	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1472.468044	2401.13	247.91	56.65
855	10.90	1725.43	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1479.056048	2404.84	251.83	56.65
860	10.90	1735.52	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1485.644052	2408.55	255.75	56.65
865	10.90	1745.61	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1492.232056	2412.26	259.67	56.65
870	10.90	1755.70	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1498.820060	2415.97	263.59	56.65
875	10.90	1765.79	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1505.408064	2419.68	267.51	56.65
880	10.90	1775.88	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1511.996068	2423.39	271.43	56.65
885	10.90	1785.97	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1518.584072	2427.10	275.35	56.65
890	10.90	1796.06	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1525.172076	2430.81	279.27	56.65
895	10.90	1806.15	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1531.760080	2434.52	283.19	56.65
900	10.90	1816.24	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1538.348084	2438.23	287.11	56.65
905	10.90	1826.33	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1544.936088	2441.94	291.03	56.65
910	10.90	1836.42	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1551.524092	2445.65	294.95	56.65
915	10.90	1846.51	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1558.112096	2449.36	298.87	56.65
920	10.90	1856.60	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1564.700100	2453.07	302.79	56.65
925	10.90	1866.69	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1571.288104	2456.78	306.71	56.65
930	10.90	1876.78	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1577.876108	2460.49	310.63	56.65
935	10.90	1886.87	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1584.464112	2464.20	314.55	56.65
940	10.90	1896.96	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1591.052116	2467.91	318.47	56.65
945	10.90	1907.05	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1597.640120	2471.62	322.39	56.65
950	10.90	1917.14	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1604.228124	2475.33	326.31	56.65
955	10.90	1927.23	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1610.816128	2479.04	330.23	56.65
960	10.90	1937.32	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1617.404132	2482.75	334.15	56.65
965	10.90	1947.41	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1623.992136	2486.46	338.07	56.65
970	10.90	1957.50	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1630.580140	2490.17	341.99	56.65
975	10.90	1967.59	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1637.168144	2493.88	345.91	56.65
980	10.90	1977.68	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1643.756148	2497.59	349.83	56.65
985	10.90	1987.77	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1650.344152	2501.30	353.75	56.65
990	10.90	1997.86	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1656.932156	2505.01	357.67	56.65
995	10.90	2007.95	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1663.520160	2508.72	361.59	56.65
1000	10.90	2018.04	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1670.108164	2512.43	365.51	56.65
1005	10.90	2028.13	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1676.696168	2516.14	369.43	56.65
1010	10.90	2038.22	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1683.284172	2519.85	373.35	56.65
1015	10.90	2048.31	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1689.872176	2523.56	377.27	56.65
1020	10.90	2058.40	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1696.460180	2527.27	381.19	56.65
1025	10.90	2068.49	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1703.048184	2530.98	385.11	56.65
1030	10.90	2078.58	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1709.636188	2534.69	389.03	56.65
1035	10.90	2088.67	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1716.224192	2538.40	392.95	56.65
1040	10.90	2098.76	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1722.812196	2542.11	396.87	56.65
1045	10.90	2108.85	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1729.400200	2545.82	400.79	56.65
1050	10.90	2118.94	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1735.988204	2549.53	404.71	56.65
1055	10.90	2129.03	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1742.576208	2553.24	408.63	56.65
1060	10.90	2139.12	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1749.164212	2556.95	412.55	56.65
1065	10.90	2149.21	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1755.752216	2560.66	416.47	56.65
1070	10.90	2159.30	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1762.340220	2564.37	420.39	56.65
1075	10.90	2169.39	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1768.928224	2568.08	424.31	56.65
1080	10.90	2179.48	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1775.516228	2571.79	428.23	56.65
1085	10.90	2189.57	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1782.104232	2575.50	432.15	56.65
1090	10.90	2199.66	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1788.692236	2579.21	436.07	56.65
1095	10.90	2209.75	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1795.280240	2582.92	439.99	56.65
1100	10.90	2219.84	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1801.868244	2586.63	443.91	56.65
1105	10.90	2229.93	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1808.456248	2590.34	447.83	56.65
1110	10.90	2240.02	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1815.044252	2594.05	451.75	56.65
1115	10.90	2250.11	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1821.632256	2597.76	455.67	56.65
1120	10.90	2260.20	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1828.220260	2601.47	459.59	56.65
1125	10.90	2270.29	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1834.808264	2605.18	463.51	56.65
1130	10.90	2280.38	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1841.396268	2608.89	467.43	56.65
1135	10.90	2290.47	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1847.984272	2612.60	471.35	56.65
1140	10.90	2300.56	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1854.572276	2616.31	475.27	56.65
1145	10.90	2310.65	0.05	0.02082492	0.0000	0.0000	0.0204	0.0004	6.176	1861.160280	2620.02	479.19	56.65



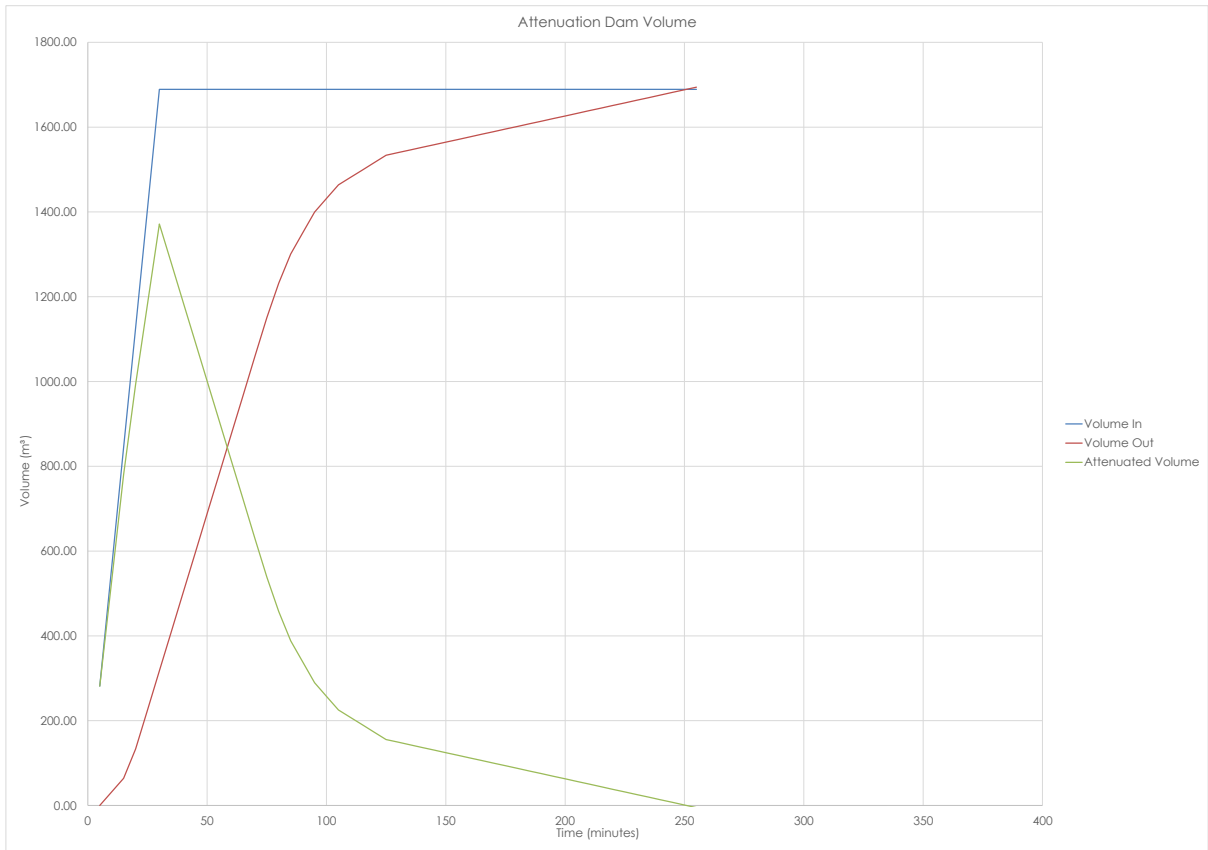
	V = Q*time increment	sum of inflow	water height of balance of flow	outlet control culvert formula	outlet overflow orifice formula	emergency overflow	Connecting to main line, max capacity, stepped, based on previous increments inflow	Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL= V _o - V _{out}	in - out	Adjusted for outflow
Time (min)	Volume In	Cumulative Volume	Water Height Above Intet	Weir Outflow	Orifice Overflow	Overflow Channel	pipe outflow	Max Outflow per Incre	Max Volume Out per increment	Cumulative Volume Out	Total Dam Volume Incl PWL	Attenuated Volume	Actual Level
5	147.483	147.48	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	0.0204	0.0204	204.04	147.48	56.55
10	147.483	294.97	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	0.0204	4.176	244.35	288.79	56.73
15	147.483	442.45	0.15	0.1070	0.0000	0.0000	0.0204	0.0204	6.176	12.352	2386.66	430.10	56.80
20	147.483	589.93	0.2	0.1447	0.0000	0.0000	0.1070	0.1070	32.591	44.442	2702.05	545.49	56.85
25	147.483	737.41	0.25	0.2000	0.0000	0.0000	0.1447	0.1447	49.407	93.649	2803.12	643.54	56.90
30	147.483	884.90	0.3	0.2704	0.0000	0.0000	0.2000	0.2000	68.993	162.842	2878.61	722.05	56.95
35	0.000	884.90	0.35	0.3059	0.0000	0.0000	0.2704	0.2704	81.184	244.024	2797.43	640.87	56.90
40	0.000	884.90	0.3	0.2704	0.0000	0.0000	0.3059	0.3059	91.749	335.773	2705.66	549.10	56.85
45	0.000	884.90	0.25	0.2000	0.0000	0.0000	0.2704	0.2704	81.184	416.957	2624.48	467.92	56.80
50	0.000	884.90	0.2	0.1447	0.0000	0.0000	0.2000	0.2000	68.993	485.972	2553.48	395.92	56.80
55	0.000	884.90	0.2	0.1447	0.0000	0.0000	0.1447	0.1447	49.407	535.379	2504.08	349.52	56.75
60	0.000	884.90	0.15	0.1070	0.0000	0.0000	0.1447	0.1447	49.407	584.786	2454.67	303.11	56.75
65	0.000	884.90	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	32.591	618.877	2424.88	268.02	56.70
70	0.000	884.90	0.1	0.0582	0.0000	0.0000	0.1070	0.1070	32.591	648.967	2394.49	232.93	56.70
75	0.000	884.90	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	666.435	2375.02	218.46	56.70
80	0.000	884.90	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	683.903	2357.55	203.99	56.70
85	0.000	884.90	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	701.371	2340.08	189.52	56.65
90	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0582	0.0582	17.468	718.839	2322.62	166.06	56.65
95	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	735.015	2314.44	159.88	56.65
100	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	751.191	2310.26	158.70	56.65
105	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	767.367	2304.09	147.53	56.65
110	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	783.543	2297.91	141.35	56.65
115	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	799.719	2291.74	135.18	56.65
120	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	815.895	2285.56	129.00	56.65
125	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	832.070	2279.39	122.83	56.65
130	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	848.246	2273.21	116.65	56.65
135	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	864.422	2267.03	110.47	56.65
140	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	880.598	2260.86	104.30	56.65
145	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	896.774	2254.68	98.12	56.65
150	0.000	884.90	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	912.950	2248.51	91.95	56.60
155	0.000	884.90	0	0.0000	0.0000	0.0000	0.0204	0.0204	6.176	929.126	2242.33	85.77	56.60
160	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	945.301	2236.15	79.59	56.60
165	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	961.477	2229.98	73.42	56.60
170	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	977.652	2223.80	67.24	56.60
175	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	993.828	2217.63	61.07	56.60
180	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1010.003	2211.45	54.89	56.60
185	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1026.179	2205.27	48.71	56.60
190	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1042.354	2199.10	42.54	56.60
195	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1058.529	2192.92	36.36	56.60
200	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1074.704	2186.75	30.19	56.60
205	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1090.879	2180.57	24.01	56.60
210	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1107.054	2174.40	17.84	56.60
215	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1123.229	2168.22	11.66	56.60
220	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1139.404	2162.04	5.48	56.60
225	0.000	884.90	0	0.0000	0.0000	0.0000	0.0000	0.0204	6.176	1155.579	2155.87	-0.69	56.55
230	0.000	884.90	-0.05	#N/A!	0.0000	0.0000	0.0000	0.0000	0.0000	1155.579	2155.87	-0.69	56.55
235	0.000	884.90	-0.05	#N/A!	0.0000	0.0000	0.0000	0.0000	0.0000	1155.579	2155.87	-0.69	56.55
240	0.000	884.90	-0.05	#N/A!	0.0000	0.0000	0.0000	0.0000	0.0000	1155.579	2155.87	-0.69	56.55
245	0.000	884.90	-0.05	#N/A!	0.0000	0.0000	0.0000	0.0000	0.0000	1155.579	2155.87	-0.69	56.55



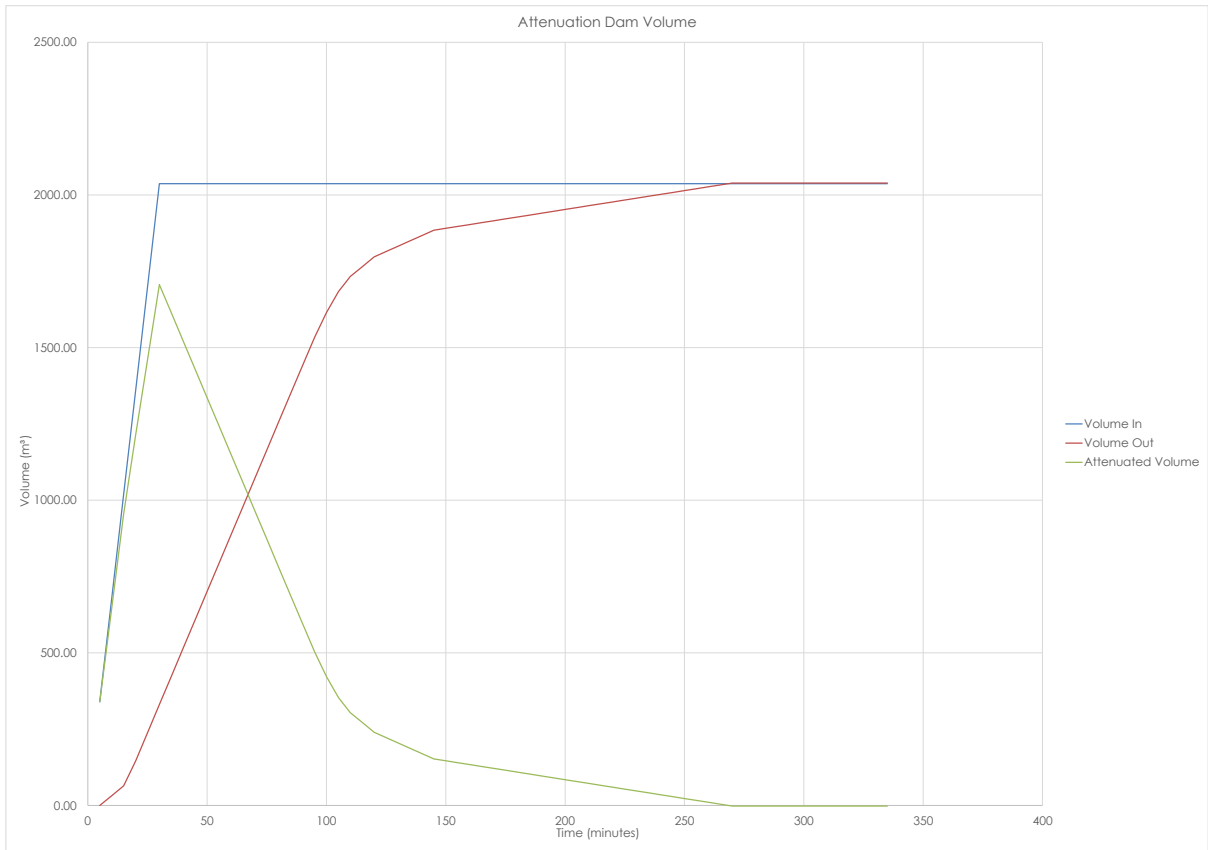
Time (min)	V = Q*time increment	sum of inflow	water height of balance of flow	outlet control culvert formula	outlet overflow/outlet formula	emergency overflow	Connecting to main line, max capacity, stepped, based on previous increments, inflow	Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL+ V _o - Vault	in - out	Adjusted for outflow
	Volume In	Cumulative Volume	Water Height Above Intet	Weir Outflow	Orifice Overflow	Overflow Channel	pipe outflow	Max Outflow per Ince	Max Volume Out per increase	Cumulative Volume Out	Total Dam Volume Incl PWL	Attenuated Volume	Actual Level
5	223.544	223.54	0.10	0.0582	0.0000	0.0000	0.0582	0.0582	0.0000	223.54	223.54	223.54	56.70
10	223.544	447.09	0.10	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	464.62	298.18	492.62	56.80
15	223.544	670.64	0.2	0.1447	0.0000	0.0000	0.0582	0.0582	17.468	689.18	2792.26	635.70	56.90
20	223.544	894.18	0.3	0.2706	0.0000	0.0000	0.1447	0.1447	49.407	913.59	2964.40	809.84	57.00
25	223.544	1117.73	0.4	0.3375	0.0000	0.0000	0.2706	0.2706	81.184	1195.07	3108.74	952.20	57.05
30	223.544	1341.28	0.45	0.3644	0.0000	0.0000	0.3375	0.3375	92.659	1387.62	3239.65	1083.09	57.10
35	0.000	1341.28	0.5	0.3932	0.0000	0.0000	0.3644	0.3644	92.659	1580.28	3346.99	1204.43	57.05
40	0.000	1341.28	0.45	0.3644	0.0000	0.0000	0.3375	0.3375	92.659	1772.93	3435.33	1317.77	57.05
45	0.000	1341.28	0.45	0.3644	0.0000	0.0000	0.3375	0.3375	92.659	1965.58	3516.67	1423.11	57.00
50	0.000	1341.28	0.4	0.3375	0.0000	0.0000	0.3089	0.3089	92.659	2158.23	3591.01	1521.45	56.95
55	0.000	1341.28	0.35	0.3059	0.0000	0.0000	0.3089	0.3089	92.659	2350.88	3659.36	1613.79	56.90
60	0.000	1341.28	0.3	0.2706	0.0000	0.0000	0.3059	0.3059	91.749	2543.53	3722.70	1701.13	56.85
65	0.000	1341.28	0.25	0.2320	0.0000	0.0000	0.2706	0.2706	81.184	2736.18	3788.04	1783.47	56.80
70	0.000	1341.28	0.2	0.1447	0.0000	0.0000	0.2320	0.2320	68.993	2928.83	3848.41	1861.81	56.75
75	0.000	1341.28	0.15	0.1070	0.0000	0.0000	0.1447	0.1447	49.407	3121.48	3904.00	1936.15	56.75
80	0.000	1341.28	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	32.991	3314.13	3951.35	2007.49	56.75
85	0.000	1341.28	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	32.991	3506.78	3990.04	2075.83	56.70
90	0.000	1341.28	0.1	0.0582	0.0000	0.0000	0.1070	0.1070	32.991	3699.43	4020.48	2141.17	56.70
95	0.000	1341.28	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	3892.08	4043.94	2204.51	56.70
100	0.000	1341.28	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	4084.73	4061.40	2265.85	56.70
105	0.000	1341.28	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	17.468	4277.38	4072.86	2325.19	56.65
110	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0582	0.0582	17.468	4470.03	4078.32	2382.53	56.65
115	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	4662.68	4078.32	2437.87	56.65
120	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	4855.33	4073.86	2491.21	56.65
125	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	5047.98	4064.40	2542.55	56.65
130	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	5240.63	4050.94	2591.89	56.65
135	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	5433.28	4033.48	2639.23	56.65
140	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	5625.93	4012.02	2684.57	56.65
145	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	5818.58	3987.56	2727.91	56.65
150	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	6011.23	3960.10	2769.25	56.65
155	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	6203.88	3929.64	2809.59	56.65
160	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	6396.53	3896.18	2848.93	56.65
165	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	6589.18	3860.72	2887.27	56.65
170	0.000	1341.28	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	6.176	6781.83	3823.26	2924.61	56.65
175	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	6974.48	3783.80	2960.95	56.60
180	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	7167.13	3742.34	2996.29	56.60
185	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	7359.78	3700.88	3030.63	56.60
190	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	7552.43	3660.42	3064.97	56.60
195	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	7745.08	3620.96	3099.31	56.60
200	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	7937.73	3582.50	3133.65	56.60
205	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	8130.38	3545.04	3167.99	56.60
210	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	8323.03	3508.58	3202.33	56.60
215	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	8515.68	3473.12	3236.67	56.60
220	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	8708.33	3438.66	3271.01	56.60
225	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	8900.98	3405.20	3305.35	56.60
230	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	9093.63	3372.74	3339.69	56.60
235	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	9286.28	3341.28	3374.03	56.60
240	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	9478.93	3310.82	3408.37	56.60
245	0.000	1341.28	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	9671.58	3281.36	3442.71	56.55



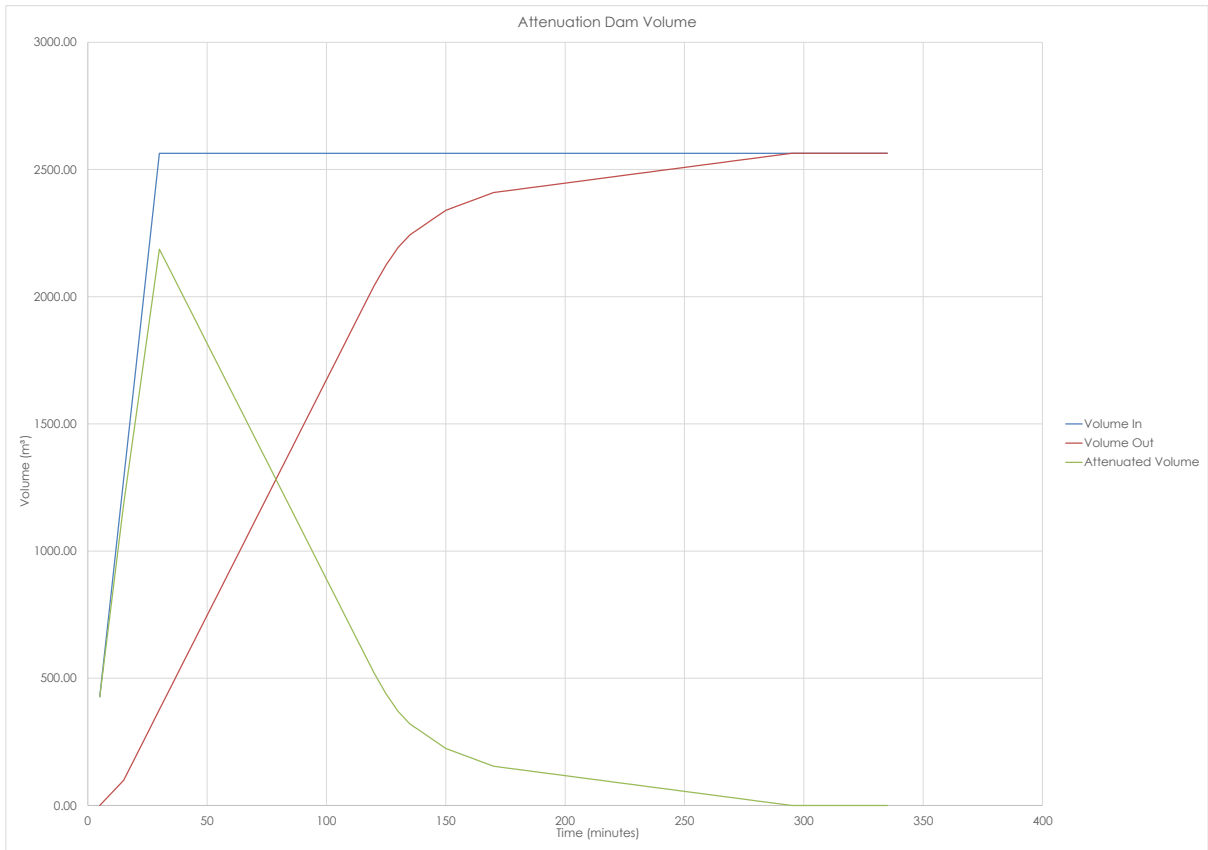
Time (min)	V = Q*time increment	sum of inflow	water height of balance of flow	outlet control culvert formula	outlet overflow orifice formula	emergency overflow	Connecting to main line, max capacity, stepped, based on previous increments inflow	Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL = V _{in} - V _{out}	in - out	Adjusted for outflow
	Volume In	Cumulative Volume	Water Height Above Intet	Weir Outflow	Orifice Overflow	Overflow Channel	pipe outflow	Max Outflow per Ince	Max Volume Out per incre	Cumulative Volume Out	Total Dam Volume Incl PWL	Attenuated Volume	Actual Level
5	281.520	281.52	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	281.52	281.52	0.0000	281.52	56.75
10	281.520	563.04	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	563.04	563.04	0.0000	563.04	56.85
15	281.520	844.56	0.25	0.2300	0.0000	0.0000	0.1070	0.1070	844.56	844.56	0.0000	844.56	56.95
20	281.520	1126.08	0.35	0.3599	0.0000	0.0000	0.2300	0.2300	1126.08	1126.08	0.0000	1126.08	57.05
25	281.520	1407.60	0.45	0.3664	0.0000	0.0000	0.3599	0.3599	1407.60	1407.60	0.0000	1407.60	57.15
30	281.520	1689.12	0.55	0.4183	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.25
35	0.000	1689.12	0.65	0.4644	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.30
40	0.000	1689.12	0.75	0.4419	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.35
45	0.000	1689.12	0.85	0.4183	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.40
50	0.000	1689.12	0.95	0.3932	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.45
55	0.000	1689.12	1.05	0.3932	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.50
60	0.000	1689.12	1.15	0.3664	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.55
65	0.000	1689.12	1.25	0.3375	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.60
70	0.000	1689.12	1.35	0.3059	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.65
75	0.000	1689.12	1.45	0.2706	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.70
80	0.000	1689.12	1.55	0.2300	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.75
85	0.000	1689.12	1.65	0.1847	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.80
90	0.000	1689.12	1.75	0.1447	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.85
95	0.000	1689.12	1.85	0.1100	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.90
100	0.000	1689.12	1.95	0.0799	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	57.95
105	0.000	1689.12	2.05	0.0582	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.00
110	0.000	1689.12	2.15	0.0428	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.05
115	0.000	1689.12	2.25	0.0322	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.10
120	0.000	1689.12	2.35	0.0250	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.15
125	0.000	1689.12	2.45	0.0206	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.20
130	0.000	1689.12	2.55	0.0184	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.25
135	0.000	1689.12	2.65	0.0167	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.30
140	0.000	1689.12	2.75	0.0154	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.35
145	0.000	1689.12	2.85	0.0144	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.40
150	0.000	1689.12	2.95	0.0136	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.45
155	0.000	1689.12	3.05	0.0130	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.50
160	0.000	1689.12	3.15	0.0125	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.55
165	0.000	1689.12	3.25	0.0121	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.60
170	0.000	1689.12	3.35	0.0118	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.65
175	0.000	1689.12	3.45	0.0115	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.70
180	0.000	1689.12	3.55	0.0113	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.75
185	0.000	1689.12	3.65	0.0111	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.80
190	0.000	1689.12	3.75	0.0110	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.85
195	0.000	1689.12	3.85	0.0109	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.90
200	0.000	1689.12	3.95	0.0108	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	58.95
205	0.000	1689.12	4.05	0.0107	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.00
210	0.000	1689.12	4.15	0.0106	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.05
215	0.000	1689.12	4.25	0.0105	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.10
220	0.000	1689.12	4.35	0.0104	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.15
225	0.000	1689.12	4.45	0.0103	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.20
230	0.000	1689.12	4.55	0.0102	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.25
235	0.000	1689.12	4.65	0.0101	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.30
240	0.000	1689.12	4.75	0.0100	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.35
245	0.000	1689.12	4.85	0.0099	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.40
250	0.000	1689.12	4.95	0.0098	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.45
255	0.000	1689.12	5.05	0.0097	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.50
260	0.000	1689.12	5.15	0.0096	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.55
265	0.000	1689.12	5.25	0.0095	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.60
270	0.000	1689.12	5.35	0.0094	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.65
275	0.000	1689.12	5.45	0.0093	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.70
280	0.000	1689.12	5.55	0.0092	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.75
285	0.000	1689.12	5.65	0.0091	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.80
290	0.000	1689.12	5.75	0.0090	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.85
295	0.000	1689.12	5.85	0.0089	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.90
300	0.000	1689.12	5.95	0.0088	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	59.95
305	0.000	1689.12	6.05	0.0087	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.00
310	0.000	1689.12	6.15	0.0086	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.05
315	0.000	1689.12	6.25	0.0085	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.10
320	0.000	1689.12	6.35	0.0084	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.15
325	0.000	1689.12	6.45	0.0083	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.20
330	0.000	1689.12	6.55	0.0082	0.0000	0.0000	0.3664	0.3664	1689.12	1689.12	0.0000	1689.12	60.25



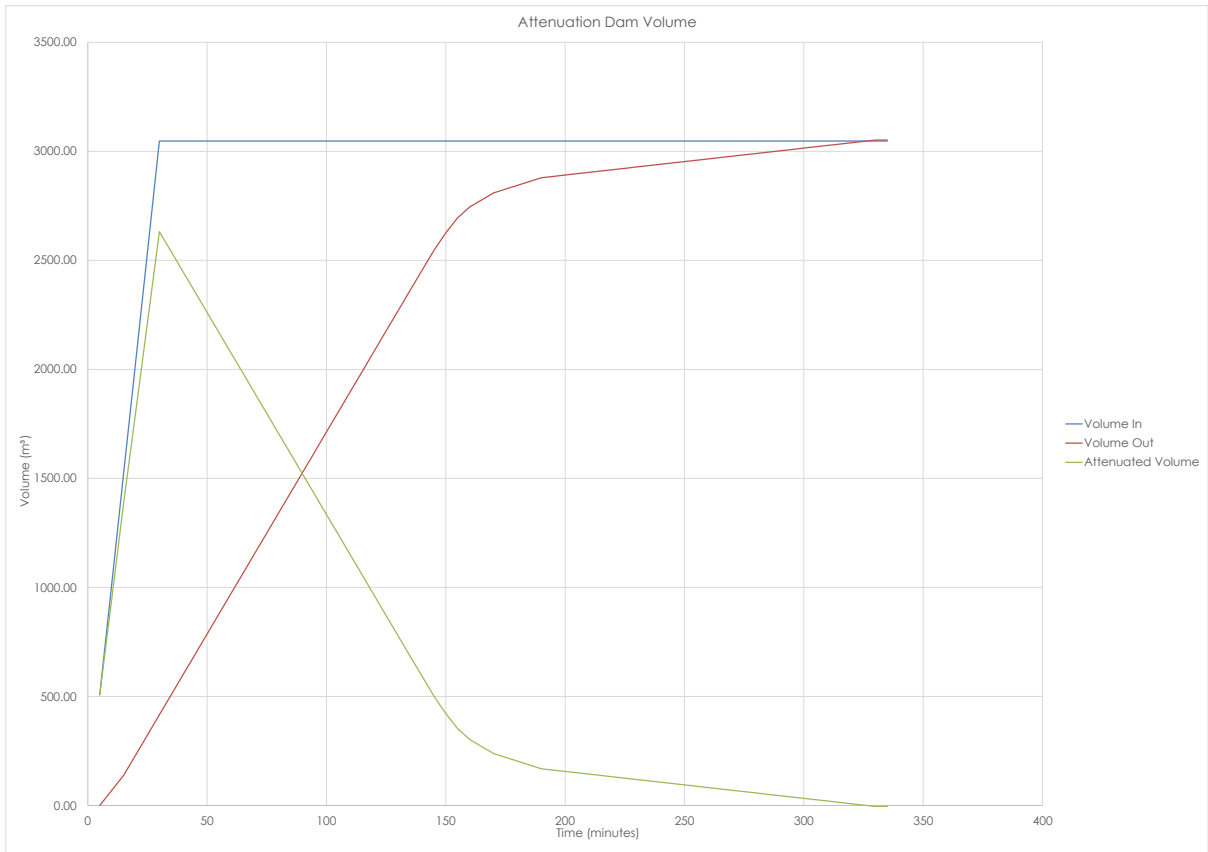
Time (min)	V = Q*time increment	sum of inflow	water height of balance of flow	outlet control culvert formula	outlet overflow orifice formula	emergency overflow	Connecting to main line, max capacity, stopped based on previous increments inflow	Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL = V _{in} - V _{out}	in - out	Adjusted for outflow
	Volume In	Cumulative Volume	Water Height Above Intet	Weir Outflow	Orifice Overflow	Overflow Channel	pipe outflow	Max Outflow per Incre	Max Volume Out per incre	Cumulative Volume Out	Total Dam Volume Incl PWL	Attenuated Volume	Actual Level
5	339.423	339.423	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	32.091	0.0000	2494.025	339.423	54.73
10	339.493	678.916	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	32.091	32.091	2833.46	678.916	56.90
15	339.493	1018.409	0.3	0.2706	0.0000	0.0000	0.1070	0.1070	32.091	64.181	3110.86	1018.409	57.02
20	339.493	1357.902	0.45	0.3644	0.0000	0.0000	0.2706	0.2706	81.184	145.365	3369.17	1357.902	57.30
25	339.493	1697.395	0.6	0.4412	0.0000	0.0000	0.3089	0.3089	92.459	238.025	3614.00	1697.395	57.30
30	339.493	2036.888	0.7	0.4858	0.0000	0.0000	0.3089	0.3089	92.459	330.484	3862.83	2036.888	57.40
35	0.000	2036.888	0.8	0.5260	0.0000	0.0000	0.3089	0.3089	92.459	423.344	3770.17	1613.61	57.35
40	0.000	2036.888	0.75	0.5043	0.0000	0.0000	0.3089	0.3089	92.459	516.003	3477.51	1520.95	57.30
45	0.000	2036.888	0.7	0.4828	0.0000	0.0000	0.3089	0.3089	92.459	608.663	3184.84	1428.31	57.25
50	0.000	2036.888	0.65	0.4644	0.0000	0.0000	0.3089	0.3089	92.459	701.322	3492.20	1335.64	57.25
55	0.000	2036.888	0.65	0.4644	0.0000	0.0000	0.3089	0.3089	92.459	793.981	3399.54	1242.98	57.20
60	0.000	2036.888	0.6	0.4419	0.0000	0.0000	0.3089	0.3089	92.459	886.641	3306.88	1150.32	57.15
65	0.000	2036.888	0.55	0.4183	0.0000	0.0000	0.3089	0.3089	92.459	979.301	3214.22	1057.66	57.10
70	0.000	2036.888	0.5	0.3932	0.0000	0.0000	0.3089	0.3089	92.459	1071.960	3121.56	965.00	57.05
75	0.000	2036.888	0.45	0.3644	0.0000	0.0000	0.3089	0.3089	92.459	1164.620	3028.90	872.34	57.00
80	0.000	2036.888	0.4	0.3373	0.0000	0.0000	0.3089	0.3089	92.459	1257.279	2936.24	779.08	56.95
85	0.000	2036.888	0.35	0.3059	0.0000	0.0000	0.3089	0.3089	92.459	1349.939	2843.58	685.82	56.95
90	0.000	2036.888	0.3	0.2699	0.0000	0.0000	0.3059	0.3059	91.749	1441.708	2751.81	592.56	56.90
95	0.000	2036.888	0.3	0.2706	0.0000	0.0000	0.3059	0.3059	91.749	1533.477	2660.04	503.48	56.85
100	0.000	2036.888	0.25	0.2300	0.0000	0.0000	0.2706	0.2706	81.184	1614.641	2578.86	422.30	56.80
105	0.000	2036.888	0.2	0.1847	0.0000	0.0000	0.2300	0.2300	48.393	1683.524	2509.86	353.30	56.75
110	0.000	2036.888	0.15	0.1370	0.0000	0.0000	0.1847	0.1847	49.407	1733.061	2466.46	303.90	56.75
115	0.000	2036.888	0.15	0.1370	0.0000	0.0000	0.1370	0.1370	32.091	1765.152	2428.37	271.81	56.70
120	0.000	2036.888	0.1	0.0962	0.0000	0.0000	0.1070	0.1070	32.091	1797.243	2396.28	239.72	56.70
125	0.000	2036.888	0.1	0.0962	0.0000	0.0000	0.0962	0.0962	17.468	1814.710	2378.81	222.25	56.70
130	0.000	2036.888	0.1	0.082	0.0000	0.0000	0.082	0.082	17.468	1832.178	2361.34	204.78	56.70
135	0.000	2036.888	0.1	0.082	0.0000	0.0000	0.082	0.082	17.468	1849.646	2343.87	187.31	56.70
140	0.000	2036.888	0.1	0.066	0.0000	0.0000	0.066	0.066	17.468	1867.114	2326.40	169.84	56.65
145	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.052	0.052	17.468	1884.582	2308.94	152.38	56.65
150	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1892.050	2302.76	146.20	56.65
155	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1896.524	2296.58	140.02	56.65
160	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1903.110	2290.41	133.85	56.65
165	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1909.284	2284.23	127.67	56.65
170	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1915.457	2278.06	121.50	56.65
175	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1921.631	2271.88	115.32	56.65
180	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1927.805	2265.71	109.14	56.65
185	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1933.979	2259.53	102.97	56.65
190	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1940.153	2253.35	96.79	56.65
195	0.000	2036.888	0.05	0.0204	0.0000	0.0000	0.0204	0.0204	6.176	1946.327	2247.18	90.62	56.60
200	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1952.501	2241.00	84.44	56.60
205	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1958.675	2234.83	78.27	56.60
210	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1964.849	2228.65	72.09	56.60
215	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1971.023	2222.47	65.91	56.60
220	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1977.197	2216.30	59.74	56.60
225	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1983.371	2210.12	53.56	56.60
230	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1989.545	2203.95	47.39	56.60
235	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	1995.719	2197.77	41.21	56.60
240	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2001.893	2191.59	35.03	56.60
245	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2008.067	2185.42	28.86	56.60
250	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2014.241	2179.24	22.68	56.60
255	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2020.415	2173.07	16.51	56.60
260	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2026.589	2166.89	10.33	56.60
265	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2032.763	2160.72	4.16	56.60
270	0.000	2036.888	0	0.0000	0.0000	0.0000	0.0000	0.0000	6.176	2038.937	2154.54	-2.02	56.55
275	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
280	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
285	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
290	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
295	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
300	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
305	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
310	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
315	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
320	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
325	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55
330	0.000	2036.888	-0.05	#NUM!	0.0000	0.0000	#NUM!	0.0000	0.0000	2038.979	2154.54	-2.02	56.55



Time (min)	V = Q*time increment		water height of balance of flow	outlet control culvert formula	outlet overflow orifice formula	emergency overflow	Connecting to main line, max capacity, stepped based on previous increments inflow		Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL+ V _o - Vault	in - out	Adjusted for outflow
	Volume In	Cumulative Volume					pipe outflow	Max Outflow per Incr						
5	427.281	427.28	0.20	0.1447	0.0000	0.0000	0.0000	0.1447	0.1447	49.407	0.0000	2583.84	427.28	56.80
10	427.281	854.56	0.20	0.1447	0.0000	0.0000	0.0000	0.1447	0.1447	49.407	49.407	2991.72	854.56	57.00
15	427.281	1281.84	0.4	0.3375	0.0000	0.0000	0.0000	0.1447	0.1447	49.407	98.814	3399.59	1281.84	57.15
20	427.281	1709.13	0.55	0.4183	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	191.473	3807.47	1709.13	57.30
25	427.281	2136.41	0.7	0.4858	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	284.133	4215.35	2136.41	57.45
30	427.281	2563.69	0.85	0.5450	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	376.792	4623.23	2563.69	57.60
35	0.000	2563.69	1	0.5983	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	469.451	5031.11	2563.69	57.75
40	0.000	2563.69	0.95	0.5983	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	562.110	5438.99	2563.69	57.90
45	0.000	2563.69	0.9	0.5450	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	654.770	5846.87	2563.69	58.05
50	0.000	2563.69	0.9	0.5450	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	747.430	6254.75	2563.69	58.20
55	0.000	2563.69	0.85	0.5450	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	840.090	6662.63	2563.69	58.35
60	0.000	2563.69	0.85	0.5450	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	932.749	7070.51	2563.69	58.50
65	0.000	2563.69	0.75	0.5063	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1025.409	7478.39	2563.69	58.65
70	0.000	2563.69	0.7	0.4858	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1118.068	7886.27	2563.69	58.80
75	0.000	2563.69	0.7	0.4858	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1210.728	8294.15	2563.69	58.95
80	0.000	2563.69	0.65	0.4644	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1303.387	8702.03	2563.69	59.10
85	0.000	2563.69	0.6	0.4419	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1396.047	9109.91	2563.69	59.25
90	0.000	2563.69	0.55	0.4183	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1488.706	9517.79	2563.69	59.40
95	0.000	2563.69	0.5	0.3932	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1581.366	9925.67	2563.69	59.55
100	0.000	2563.69	0.45	0.3664	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1674.025	10333.55	2563.69	59.70
105	0.000	2563.69	0.45	0.3664	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1766.685	10741.43	2563.69	59.85
110	0.000	2563.69	0.4	0.3375	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1859.344	11149.31	2563.69	60.00
115	0.000	2563.69	0.35	0.3089	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	1952.004	11557.19	2563.69	60.15
120	0.000	2563.69	0.3	0.2796	0.0000	0.0000	0.0000	0.3089	0.3089	92.659	2044.663	11965.07	2563.69	60.30
125	0.000	2563.69	0.25	0.2320	0.0000	0.0000	0.0000	0.2704	0.2704	81.184	2137.323	12372.95	2563.69	60.45
130	0.000	2563.69	0.2	0.1847	0.0000	0.0000	0.0000	0.2300	0.2300	68.993	2230.000	12780.83	2563.69	60.60
135	0.000	2563.69	0.15	0.1470	0.0000	0.0000	0.0000	0.1847	0.1847	49.407	2322.659	13188.71	2563.69	60.75
140	0.000	2563.69	0.15	0.1470	0.0000	0.0000	0.0000	0.1847	0.1847	49.407	2415.318	13596.59	2563.69	60.90
145	0.000	2563.69	0.15	0.1470	0.0000	0.0000	0.0000	0.1847	0.1847	49.407	2507.977	14004.47	2563.69	61.05
150	0.000	2563.69	0.1	0.0982	0.0000	0.0000	0.0000	0.1070	0.1070	32.091	2599.636	14412.35	2563.69	61.20
155	0.000	2563.69	0.1	0.0982	0.0000	0.0000	0.0000	0.0982	0.0982	17.468	2691.295	14820.23	2563.69	61.35
160	0.000	2563.69	0.1	0.0982	0.0000	0.0000	0.0000	0.0982	0.0982	17.468	2782.954	15228.11	2563.69	61.50
165	0.000	2563.69	0.1	0.0982	0.0000	0.0000	0.0000	0.0982	0.0982	17.468	2874.613	15635.99	2563.69	61.65
170	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0582	0.0582	17.468	2966.272	16043.87	2563.69	61.80
175	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3057.931	16451.75	2563.69	61.95
180	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3149.590	16859.63	2563.69	62.10
185	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3241.249	17267.51	2563.69	62.25
190	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3332.908	17675.39	2563.69	62.40
195	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3424.567	18083.27	2563.69	62.55
200	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3516.226	18491.15	2563.69	62.70
205	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3607.885	18899.03	2563.69	62.85
210	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3699.544	19306.91	2563.69	63.00
215	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3791.203	19714.79	2563.69	63.15
220	0.000	2563.69	0.05	0.0206	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3882.862	20122.67	2563.69	63.30
225	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	3974.521	20530.55	2563.69	63.45
230	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4066.180	20938.43	2563.69	63.60
235	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4157.839	21346.31	2563.69	63.75
240	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4249.498	21754.19	2563.69	63.90
245	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4341.157	22162.07	2563.69	64.05
250	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4432.816	22569.95	2563.69	64.20
255	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4524.475	22977.83	2563.69	64.35
260	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4616.134	23385.71	2563.69	64.50
265	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4707.793	23793.59	2563.69	64.65
270	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4799.452	24201.47	2563.69	64.80
275	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4891.111	24609.35	2563.69	64.95
280	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	4982.770	25017.23	2563.69	65.10
285	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	5074.429	25425.11	2563.69	65.25
290	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	5166.088	25832.99	2563.69	65.40
295	0.000	2563.69	0	0.0000	0.0000	0.0000	0.0000	0.0206	0.0206	6.176	5257.747	26240.87	2563.69	65.55
300	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
305	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
310	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
315	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
320	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
325	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55
330	0.000	2563.69	-0.05	#N/A	0.0000	0.0000	0.0000	0.0000	0.0000	0.000	2563.69	2156.35	-0.21	56.55



Time (min)	V = Q*time increment		water height of balance of flow	outlet control culvert formula	outlet overflow orifice formula	emergency overflow	Connecting to main line, max capacity, stepped based on previous increments, inflow		Combined outlets outflow	total volume out per increment, governed by minimum capacity	Sum of volume out	PWL+ V _o - V _{out}	in - out	Adjusted for outflow
	Volume In	Cumulative Volume					pipe outflow	Max Outflow per Incr						
5	507.894	507.894	0.25	0.2900	0.0000	0.0000	0.2900	0.2900	0.2900	507.894	0.0000	2644.44	507.90	56.85
10	507.894	1015.79	0.25	0.2900	0.0000	0.0000	0.2900	0.2900	0.2900	1015.79	0.0000	3103.36	1015.80	57.05
15	507.894	1523.68	0.45	0.3644	0.0000	0.0000	0.3644	0.3644	0.3644	1523.68	0.0000	3562.28	1523.70	57.25
20	507.894	2031.58	0.65	0.4444	0.0000	0.0000	0.4444	0.4444	0.4444	2031.58	0.0000	4021.20	2031.60	57.45
25	507.894	2539.48	0.85	0.5400	0.0000	0.0000	0.5400	0.5400	0.5400	2539.48	0.0000	4480.12	2539.50	57.65
30	507.894	3047.38	1.1	0.5983	0.0000	0.0000	0.5983	0.5983	0.5983	3047.38	0.0000	4939.04	3047.40	57.85
35	0.000	3047.38	1.15	0.6473	0.1310	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	5397.96	3047.40	58.05
40	0.000	3047.38	1.15	0.6473	0.1310	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	5856.88	3047.40	58.25
45	0.000	3047.38	1.1	0.6314	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	6315.80	3047.40	58.45
50	0.000	3047.38	1.05	0.6151	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	6774.72	3047.40	58.65
55	0.000	3047.38	1.0	0.5983	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	7233.64	3047.40	58.85
60	0.000	3047.38	1.0	0.5983	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	7692.56	3047.40	59.05
65	0.000	3047.38	0.95	0.5811	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	8151.48	3047.40	59.25
70	0.000	3047.38	0.9	0.5633	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	8610.40	3047.40	59.45
75	0.000	3047.38	0.85	0.5450	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	9069.32	3047.40	59.65
80	0.000	3047.38	0.85	0.5450	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	9528.24	3047.40	59.85
85	0.000	3047.38	0.8	0.5269	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	9987.16	3047.40	60.05
90	0.000	3047.38	0.75	0.5063	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	10446.08	3047.40	60.25
95	0.000	3047.38	0.7	0.4858	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	10905.00	3047.40	60.45
100	0.000	3047.38	0.65	0.4644	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	11363.92	3047.40	60.65
105	0.000	3047.38	0.65	0.4644	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	11822.84	3047.40	60.85
110	0.000	3047.38	0.6	0.4419	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	12281.76	3047.40	61.05
115	0.000	3047.38	0.55	0.4183	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	12740.68	3047.40	61.25
120	0.000	3047.38	0.5	0.3932	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	13199.60	3047.40	61.45
125	0.000	3047.38	0.45	0.3664	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	13658.52	3047.40	61.65
130	0.000	3047.38	0.4	0.3375	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	14117.44	3047.40	61.85
135	0.000	3047.38	0.35	0.3059	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	14576.36	3047.40	62.05
140	0.000	3047.38	0.35	0.3059	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	15035.28	3047.40	62.25
145	0.000	3047.38	0.3	0.2706	0.0000	0.0000	0.3999	0.3999	0.3999	3047.38	0.0000	15494.20	3047.40	62.45
150	0.000	3047.38	0.25	0.2300	0.0000	0.0000	0.2706	0.2706	0.2706	3047.38	0.0000	15953.12	3047.40	62.65
155	0.000	3047.38	0.2	0.1847	0.0000	0.0000	0.2300	0.2300	0.2300	3047.38	0.0000	16412.04	3047.40	62.85
160	0.000	3047.38	0.15	0.1070	0.0000	0.0000	0.1847	0.1847	0.1847	3047.38	0.0000	16870.96	3047.40	63.05
165	0.000	3047.38	0.15	0.1070	0.0000	0.0000	0.1070	0.1070	0.1070	3047.38	0.0000	17329.88	3047.40	63.25
170	0.000	3047.38	0.1	0.0582	0.0000	0.0000	0.1070	0.1070	0.1070	3047.38	0.0000	17788.80	3047.40	63.45
175	0.000	3047.38	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	0.0582	3047.38	0.0000	18247.72	3047.40	63.65
180	0.000	3047.38	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	0.0582	3047.38	0.0000	18706.64	3047.40	63.85
185	0.000	3047.38	0.1	0.0582	0.0000	0.0000	0.0582	0.0582	0.0582	3047.38	0.0000	19165.56	3047.40	64.05
190	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0582	0.0582	0.0582	3047.38	0.0000	19624.48	3047.40	64.25
195	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	20083.40	3047.40	64.45
200	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	20542.32	3047.40	64.65
205	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	21001.24	3047.40	64.85
210	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	21460.16	3047.40	65.05
215	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	21919.08	3047.40	65.25
220	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	22378.00	3047.40	65.45
225	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	22836.92	3047.40	65.65
230	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	23295.84	3047.40	65.85
235	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	23754.76	3047.40	66.05
240	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	24213.68	3047.40	66.25
245	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	24672.60	3047.40	66.45
250	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	25131.52	3047.40	66.65
255	0.000	3047.38	0.05	0.0206	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	25590.44	3047.40	66.85
260	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0206	0.0206	0.0206	3047.38	0.0000	26049.36	3047.40	67.05
265	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	26508.28	3047.40	67.25
270	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	26967.20	3047.40	67.45
275	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	27426.12	3047.40	67.65
280	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	27885.04	3047.40	67.85
285	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	28343.96	3047.40	68.05
290	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	28802.88	3047.40	68.25
295	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	29261.80	3047.40	68.45
300	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	29720.72	3047.40	68.65
305	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	30179.64	3047.40	68.85
310	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	30638.56	3047.40	69.05
315	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	31097.48	3047.40	69.25
320	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	31556.40	3047.40	69.45
325	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	32015.32	3047.40	69.65
330	0.000	3047.38	0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	3047.38	0.0000	32474.24	3047.40	69.85



ATTENUATION FACILITY AND SWALE CAPACITY: CIVIL DESIGNER OUTPUT

Dam			
Cumulative Volume	Increment Volume	Level	Area
0	0	55	0
23.83	23.83	55.05	953.28
72.13	48.3	55.1	978.72
121.71	49.58	55.15	1004.34
172.57	50.86	55.2	1030.14
224.73	52.16	55.25	1056.14
278.19	53.46	55.3	1082.32
332.96	54.77	55.35	1108.68
389.06	56.1	55.4	1135.23
446.49	57.43	55.45	1161.97
505.26	58.77	55.5	1188.9
565.39	60.12	55.55	1216.01
626.87	61.48	55.6	1243.31
689.72	62.85	55.65	1270.8
753.95	64.23	55.7	1298.47
819.57	65.62	55.75	1326.33
886.59	67.02	55.8	1354.38
955.02	68.42	55.85	1382.61
1024.86	69.84	55.9	1411.03
1096.12	71.27	55.95	1439.63
1168.83	72.7	56	1468.42
1242.97	74.15	56.05	1497.4
1318.57	75.6	56.1	1526.57
1395.63	77.06	56.15	1555.92
1474.17	78.53	56.2	1585.45
1554.18	80.02	56.25	1615.18
1635.69	81.51	56.3	1645.09
1718.69	83.01	56.35	1675.19
1803.21	84.52	56.4	1705.47
1889.25	86.03	56.45	1735.94
1976.81	87.56	56.5	1766.6
2065.91	89.1	56.55	1797.44
2156.56	90.65	56.6	1828.47
2248.76	92.2	56.65	1859.69
2342.53	93.77	56.7	1891.09
2437.88	95.34	56.75	1922.68
2534.81	96.93	56.8	1954.46
2633.33	98.52	56.85	1986.42
2733.45	100.13	56.9	2018.57
2835.19	101.74	56.95	2050.91
2938.55	103.36	57	2083.43
3043.54	104.99	57.05	2116.14
3150.17	106.63	57.1	2149.04
3258.45	108.28	57.15	2182.12
3368.39	109.94	57.2	2215.39
3479.99	111.61	57.25	2248.84
3593.27	113.28	57.3	2282.49
3708.24	114.97	57.35	2316.32
3824.91	116.67	57.4	2350.33
3943.28	118.37	57.45	2384.53
4063.37	120.09	57.5	2418.92
4185.18	121.81	57.55	2453.5
4308.72	123.54	57.6	2488.26
4434.01	125.29	57.65	2523.21
4561.05	127.04	57.7	2558.34
4689.85	128.8	57.75	2593.66
4820.42	130.57	57.8	2629.17
4952.77	132.35	57.85	2664.86
5086.91	134.14	57.9	2700.75

PWL, 1.5m depth

Outlet Structure Overflow

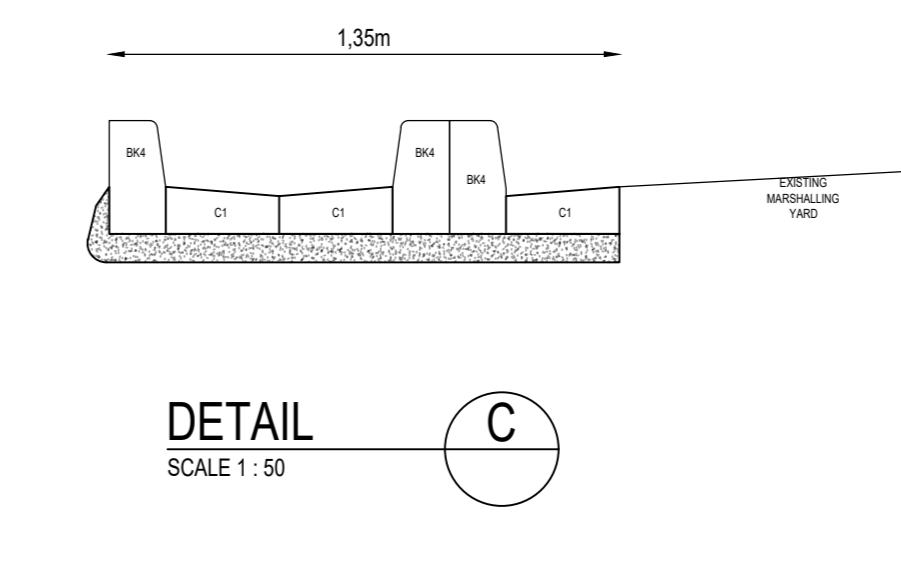
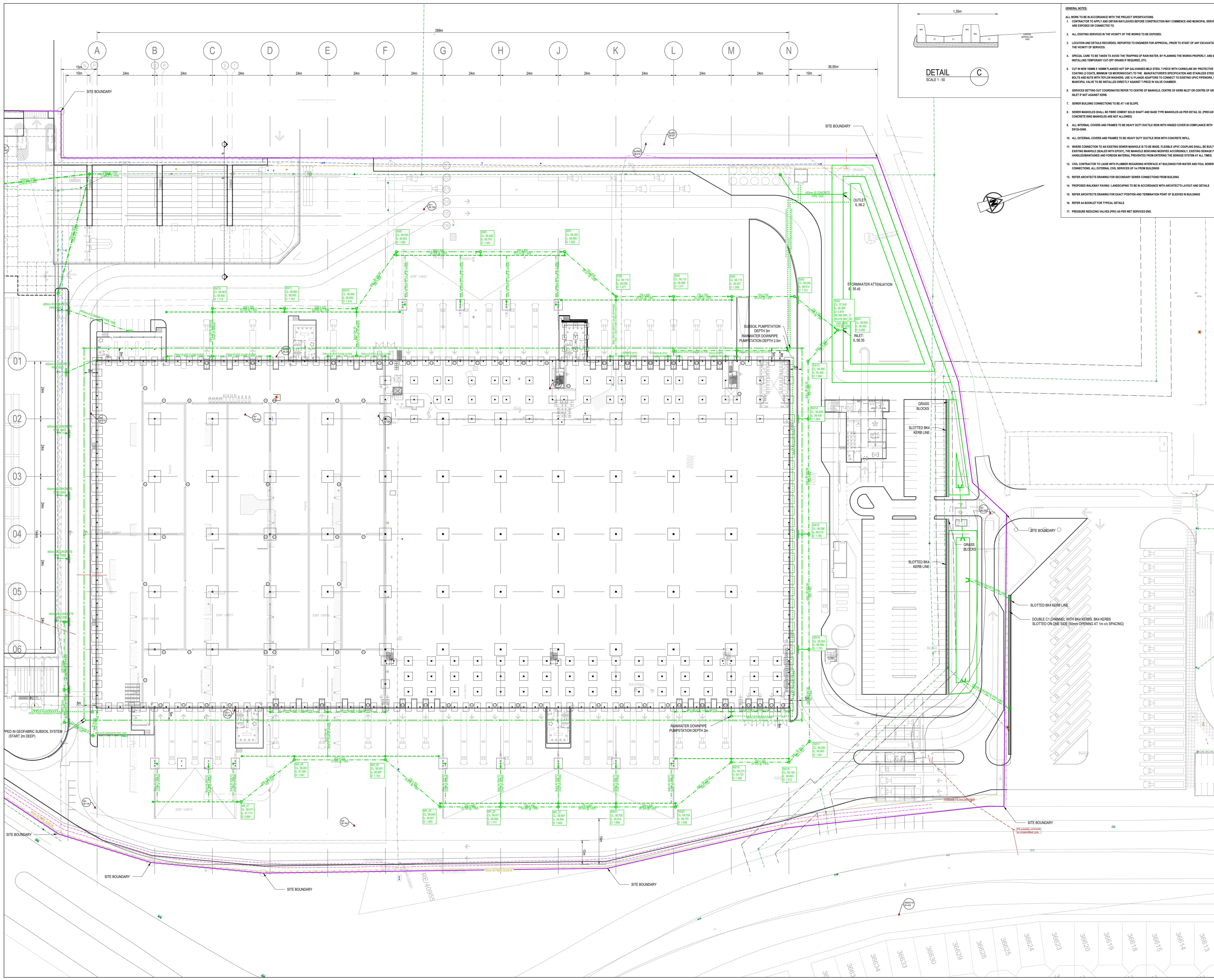
Emergency Overflow

Swale West			
Cumulative V	Increment Volume	Level	Area
0	0	57.2	22
1.788	1.788	57.3	35.768
6.234	4.446	57.4	53.147
12.584	6.35	57.5	73.857
21.172	8.588	57.6	97.898
32.331	11.158	57.7	125.27
46.393	14.062	57.8	155.972
63.692	17.299	57.9	190.005

Swale East			
Cumulative V	Increment Volume	Level	Area
0	0	57.2	337
19.404	19.404	57.3	388.085
60.829	41.425	57.4	440.415
107.551	46.722	57.5	494.028
159.699	52.148	57.6	548.923
217.4	57.701	57.7	605.101
280.783	63.383	57.8	662.56
349.976	69.193	57.9	721.302

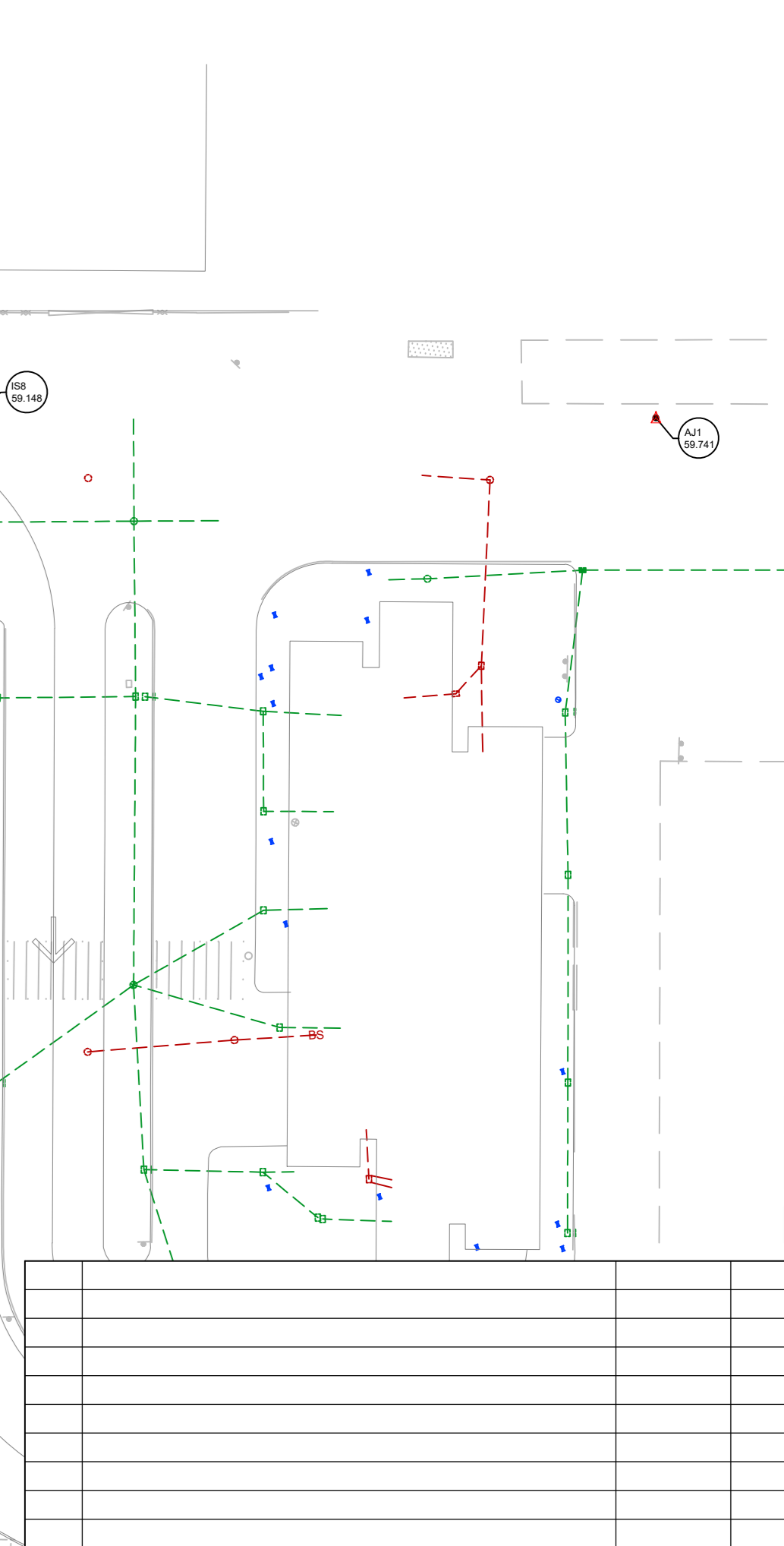
Appendix E

Stormwater Layout Plan



- GENERAL NOTES:**
- ALL WORK TO BE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
 - CONTRACTOR TO APPLY AND OBTAIN NECESSARY MUNICIPAL SERVICES BEFORE CONSTRUCTION WAY COMMENCE AND MUNICIPAL SERVICES ARE EXPOSED OR CONNECTED TO.
 - ALL EXISTING SERVICES IN THE VICINITY OF THE WORKS TO BE EXPOSED.
 - LOCATION AND DETAILS RECORDED, REPORTED TO ENGINEER FOR APPROVAL, PRIOR TO START OF ANY EXCAVATION IN THE VICINITY OF SERVICES.
 - SPECIAL CARE TO BE TAKEN TO AVOID THE TRAPPING OF RAIN WATER, BY PLANNING THE WORKS PROPERLY, AND BY INSTALLING TEMPORARY CUT-OFF DRAINS IF REQUIRED, ETC.
 - CUT IN NEW 150mm x 150mm FLANGED HOT DIP GALVANIZED WILD STEEL 3 PICES WITH CARBONACEOUS PROTECTIVE COATING (3 COATS), MINIMUM 125 MICRON THICKNESS) TO THE MANUFACTURER'S SPECIFICATION AND STAINLESS STEEL SOLE'S AND NUTS WITH TEPLOX WASHERS. USE 1/2" FLANGE ADAPTERS TO CONNECT TO EXISTING UPVC PIPEWORK. NEW MUNICIPAL VALVE TO BE INSTALLED DIRECTLY AGAINST 1 PICE IN VALVE CHAMBER.
 - SERVICES SETTING OUT COORDINATES REFER TO CENTRE OF MANHOLE, CENTRE OF KERB INLET OR CENTRE OF GRID INLET IF NOT AGAINST KERB.
 - SEWER MANHOLES CONNECTIONS TO BE AT 1:80 SLOPE.
 - SEWER MANHOLES SHALL BE FIBRE CEMENT SOLID SHAFT AND BASE TYPE MANHOLES AS PER DETAIL 12. (PRECAST CONCRETE RING MANHOLES ARE NOT ALLOWED)
 - ALL INTERNAL COVERS AND FRAMES TO BE HEAVY DUTY DUCTILE IRON WITH HINGED COVER IN COMPLIANCE WITH ENDSPEC.
 - ALL EXTERNAL COVERS AND FRAMES TO BE HEAVY DUTY DUCTILE IRON WITH CONCRETE WELLS.
 - WHERE CONNECTION TO AN EXISTING SEWER MANHOLE IS TO BE MADE, FLEXIBLE UPVC COUPLING SHALL BE BUILT INTO EXISTING MANHOLE SEALED WITH EPOXY. THE MANHOLE BEHINDING WOODEN ACCORDINGLY. EXISTING SEWAGE FLOW HANDLED/WAIVED AND FOREBODEN MATERIAL PREVENTED FROM ENTERING THE SEWAGE SYSTEM AT ALL TIMES.
 - CIVIL CONTRACTOR TO LIAISE WITH PLUMBER REGARDING INTERFACE AT BUILDINGS FOR WATER AND FOUL SEWER CONNECTIONS. ALL EXTERNAL CIVIL SERVICES UP TO FROM BUILDINGS.
 - REFER ARCHITECTS DRAWING FOR SECONDARY SEWER CONNECTIONS FROM BUILDING.
 - PROPOSED WALKWAY PAVING / LANDSCAPING TO BE IN ACCORDANCE WITH ARCHITECT'S LAYOUT AND FOOT.
 - REFER ARCHITECTS DRAWING FOR EXACT POSITION AND TERMINATION POINT OF SLEEVES IN BUILDINGS.
 - REFER AS BULLET FOR TYPICAL DETAILS.
 - PRESSURE REDUCING VALVES (PRV) AS PER WET SERVICES ENG.

- LEGEND**
- ACC 0mm 355 & ACC 0mm Junction Boxes
 - STORMWATER LINE (EPDM) & SOCKET CLASS 1800 (20mm L x 100mm O.D. (CHIMNEY SYSTEM))
 - 50mm MANHOLE CHAMBER, COVER, INVERT LEVEL & DEPTH
 - 50mm OUTLET STRUCTURE
 - 150mm KERB, INSTALLED IN GEOTEXTILE WITH HOODING EYE
 - RAINWATER DOWNPIPE (ANASTOMOZING CONNECTIONS)
 - DOUBLE GRID INLET
 - TRIPLE GRID INLET
 - 150mm FOLLIERER (UPVC Heavy Duty 30)
 - FOLLIERER MANHOLE (NUMBER, COVER, INVERT LEVEL & DEPTH)
 - 150mm FOLLIERER CONNECTION (UPVC Heavy Duty 30)
 - ROOFDRAIN EYE
 - 200mm UPVC CLASS 180 SPRING RIGID MANHOLE (CHIMNEY SYSTEM)
 - 75mm UPVC CLASS 12 WATERMAIN (ANASTOMOZING SYSTEM)
 - 40mm VALVE & CHAMBER
 - GATE VALVE & CHAMBER
 - GATE VALVE WITH HAND WHEEL IN CHAMBER
 - NON RETURN VALVE (AS PER DETAIL)
 - 150mm 50mm WATER MAIN ON 150mm 2" UPVC CLASS 18 PIPE
 - FIRE HYDRANT & CHAMBER
 - TWIN BOOSTER CONNECTOR (AS PER DETAIL)
 - 150mm UPVC CLASS 18 DOMESTIC AND FIRE WATERLINES (ANASTOMOZING SYSTEM)
 - BARRIER KERB (BKA) 150 x 40 BKA 1 (Fg 3)
 - BARRIER KERB & CHANNEL (BKA) Fg 3 OF BKA Fg 3 - C1 (Fg 14)
 - 50mm 2" UPVC CLASS 18 RAINWATER & SUBSOIL RISING MAIN
 - V-CHANNEL (2" x C1) Fg 14
 - OPEN CHANNEL WITH IMPERFOR BERM CUT
 - L11 Tariff 1000 Box
 - EXISTING SEWER & MANHOLE
 - EXISTING WATERMAIN
 - EXISTING ELECTRICAL
 - EXISTING STORMWATER LINE
 - EXISTING GREY WATER
 - LEASE BOUNDARY
 - SURVEY CONTROL LINE
 - 0.1m DESIGN CONTOURS
 - NGL CONTOURS
 - R.5. FULL CONTAINER (10 TON AXLE LOAD) - REINFORCED CONCRETE (100 x 100mm x 200mm TOP & 100mm x 100mm IN BOTTOM)
 - WALKWAY - EMPTY CONTAINER (200mm x 100mm x 100mm) - 4 TON + 40 TON AXLE LOAD - REINFORCED CONCRETE (PARK 30)
 - R.5. EMPTY CONTAINER (100mm x 100mm x 100mm) - 4 TON + 40 TON AXLE LOAD - REINFORCED CONCRETE (PARK 30)
 - CONCRETE SLABS



FOR TENDER		30/05/2023	N.W.
DATE	DATE	DATE	DATE
24/04/2023	24/04/2023	24/04/2023	24/04/2023
REVISION / WYSIGING			

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progress delivered

MAERSK BELCON LOGISTICS HUB BELLVILLE SOUTH

STORMWATER LAYOUT

APPROVED BY	SIGNATURE	PROF. NUMBER	DATE
DRG. No. 22093/502	N. WOLTER	H. KOTZE	2023-01-25
TEK. No.	CHECKED	DATE	SCALE
	MAGEZAY	24/04/2023	1:500 (A0)
			REVISION No. 1

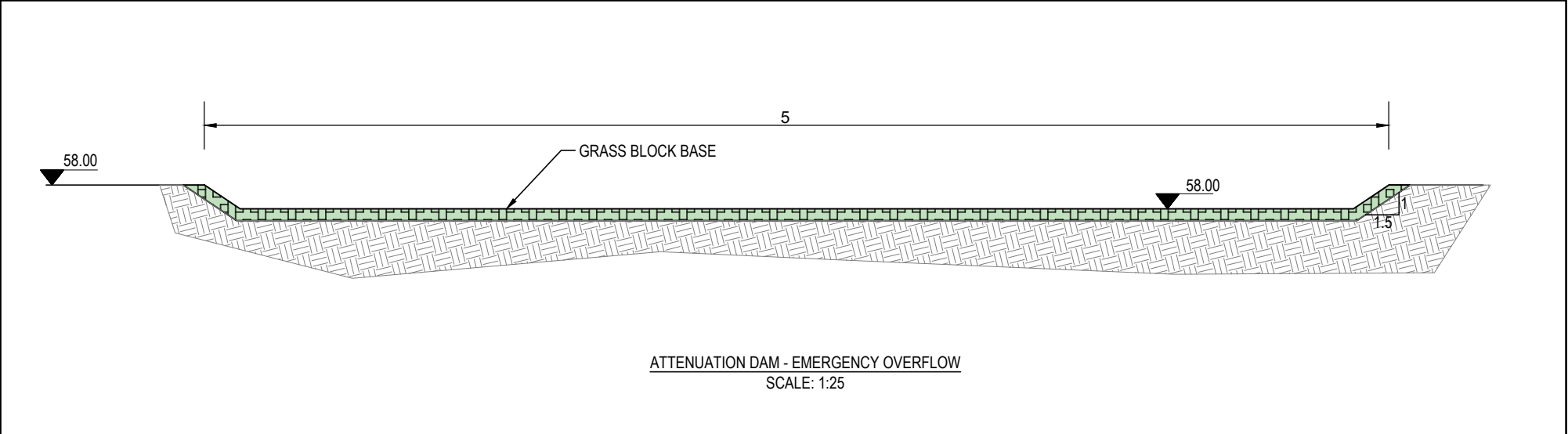
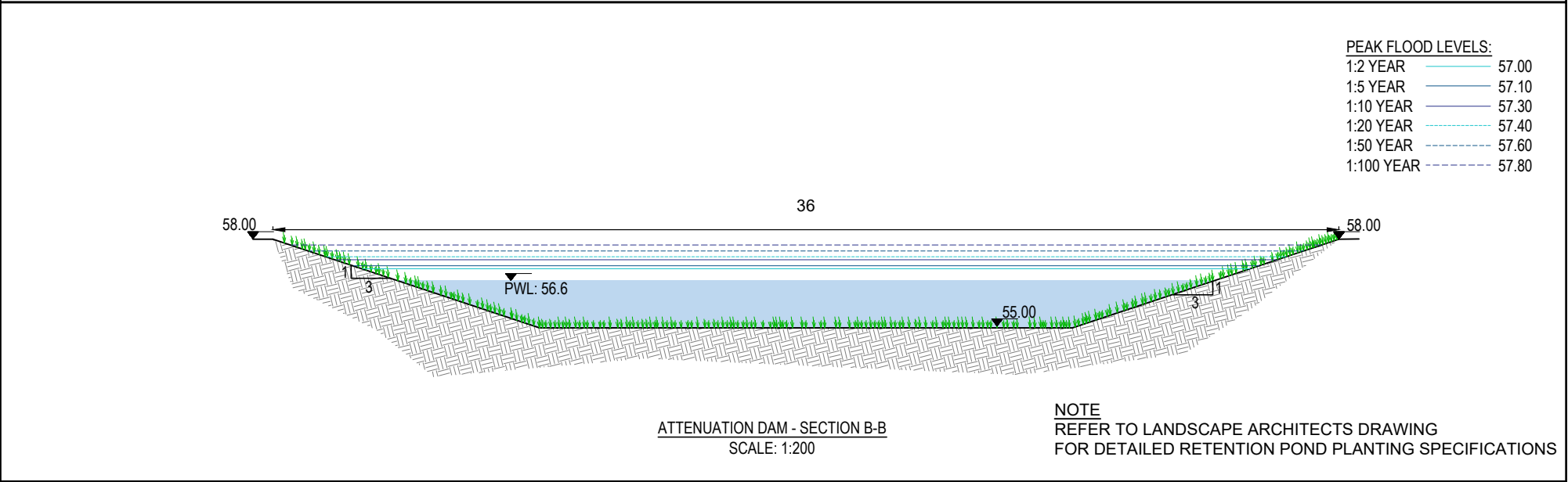
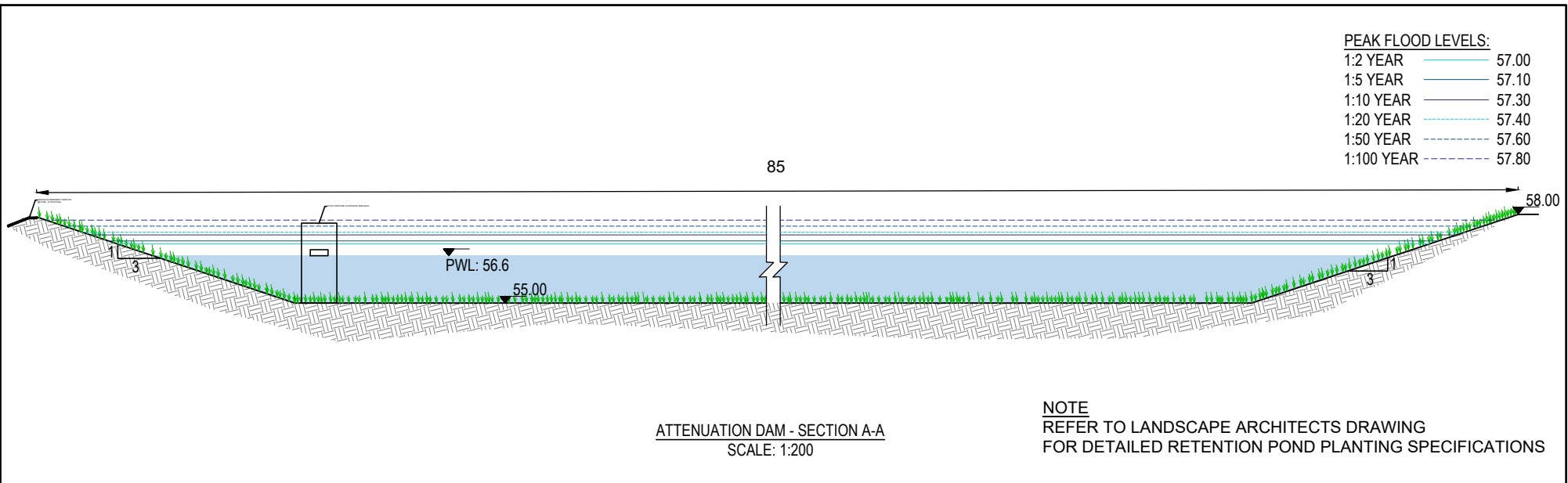
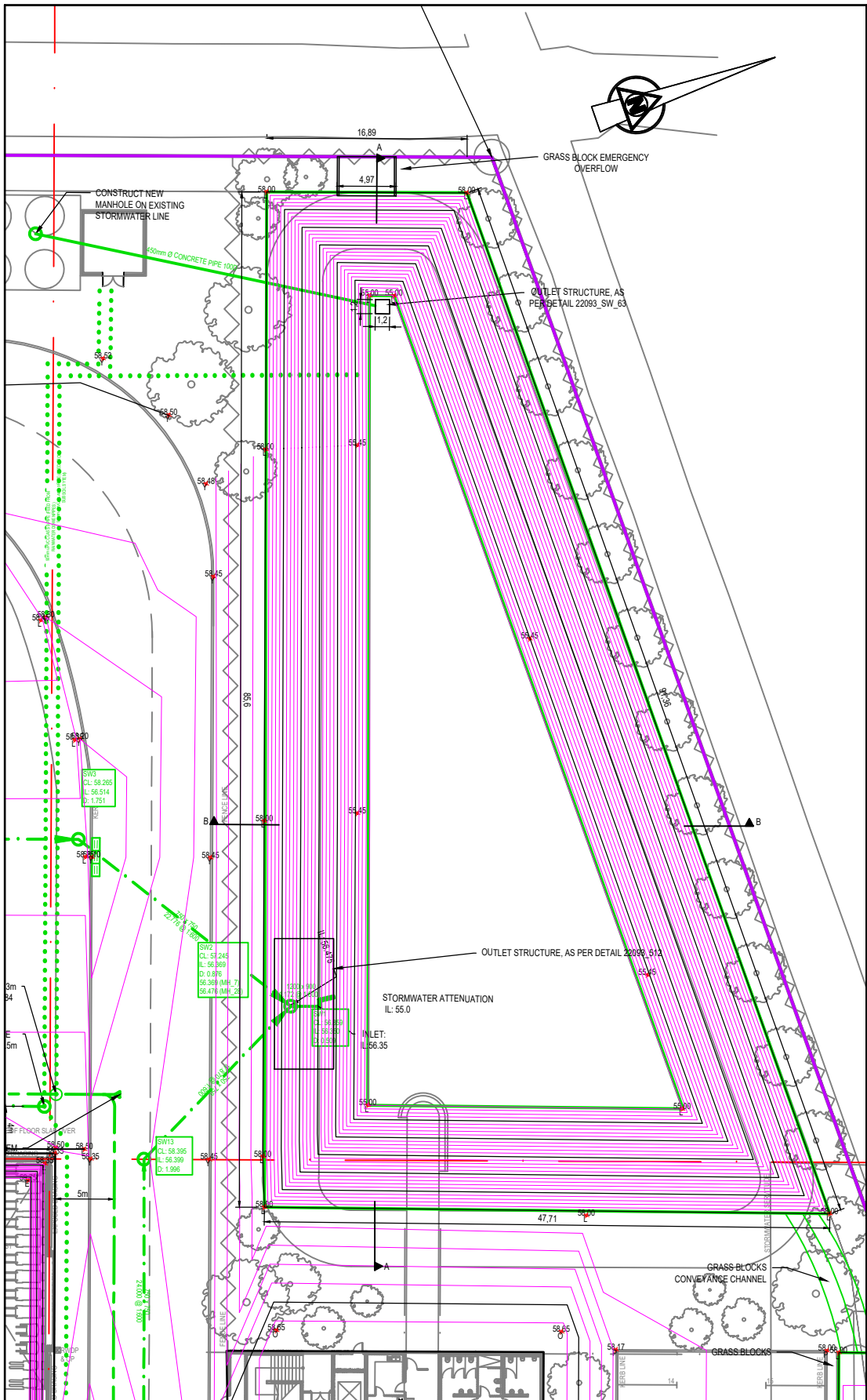
Appendix F

F1. Dam Layout and Sections

F2. Stilling Basin Typical Detail

F3. Attenuation Facility Outlet Structure

F4. Swale Typical Detail



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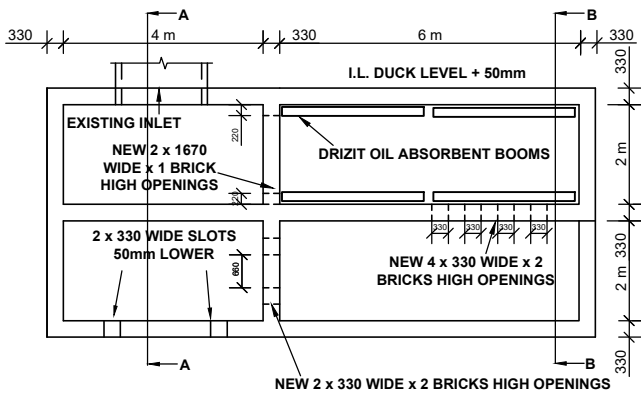
13 Pasita Street
 Rosenpark, Cape Town 7550

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 Gauteng: +27(0)12 346 1672
 Website: www.kls.co.za
 e-mail: info@kls.co.za

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ATTENUATION DAM LAYOUT AND SECTIONS

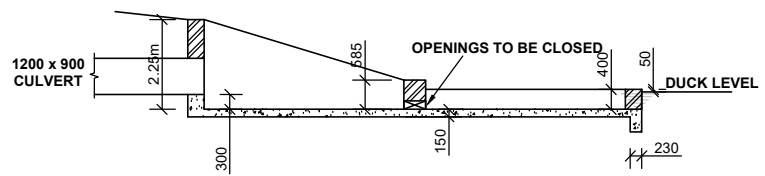
TEKENING Nr. 22093-510
 DRAWING No.



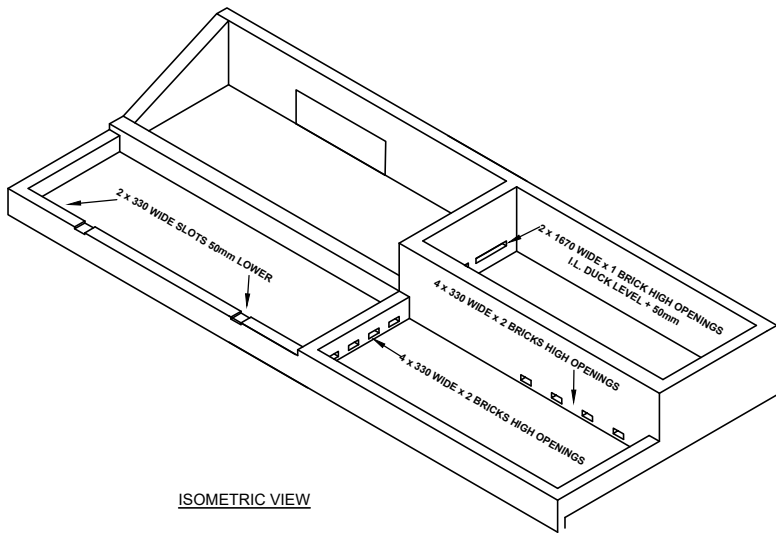
PLAN

NOTES:

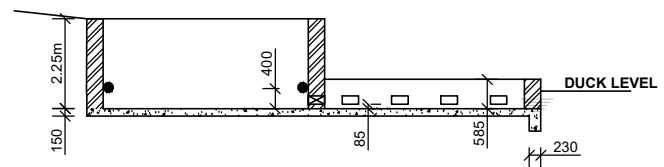
1. 330mm CAVITY WALL WITH REF 395 MESH AND CONCRETE INFILL
2. 25 MPa CONCRETE BASE WITH REF 395 MESH (TOP & BOTTOM)
3. WALL TO BE PLASTERED



SECTION A - A

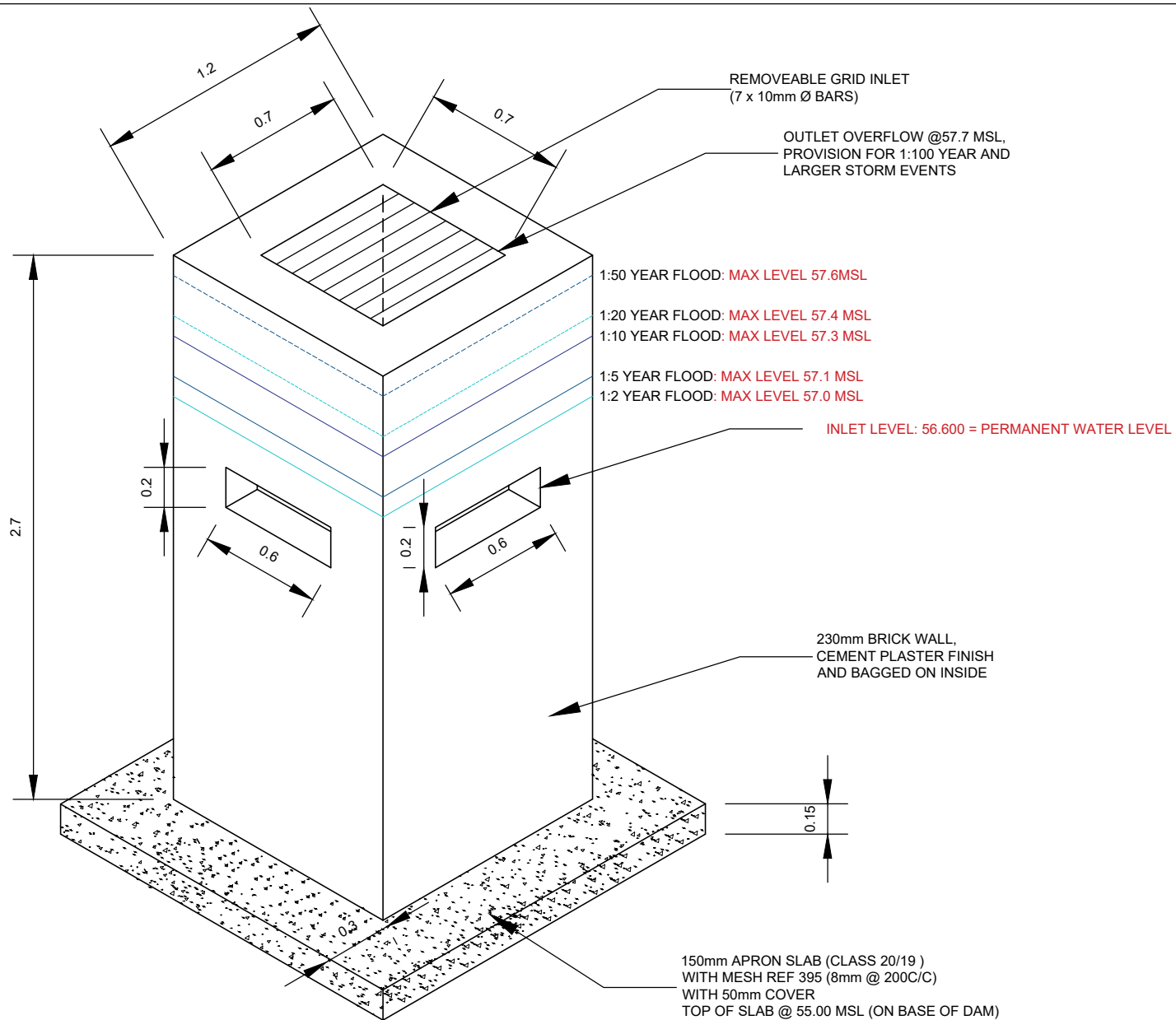


ISOMETRIC VIEW

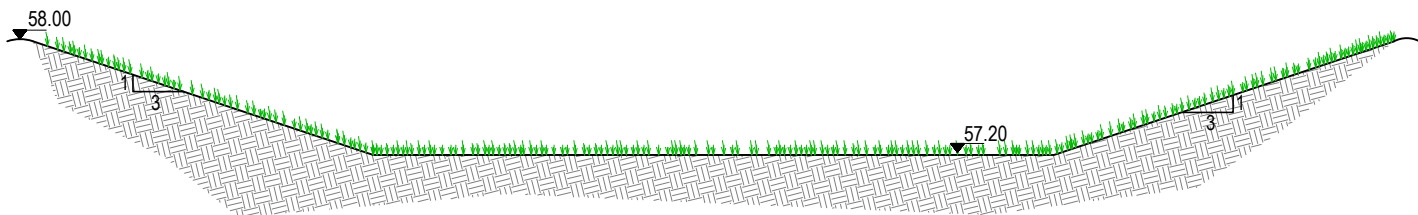
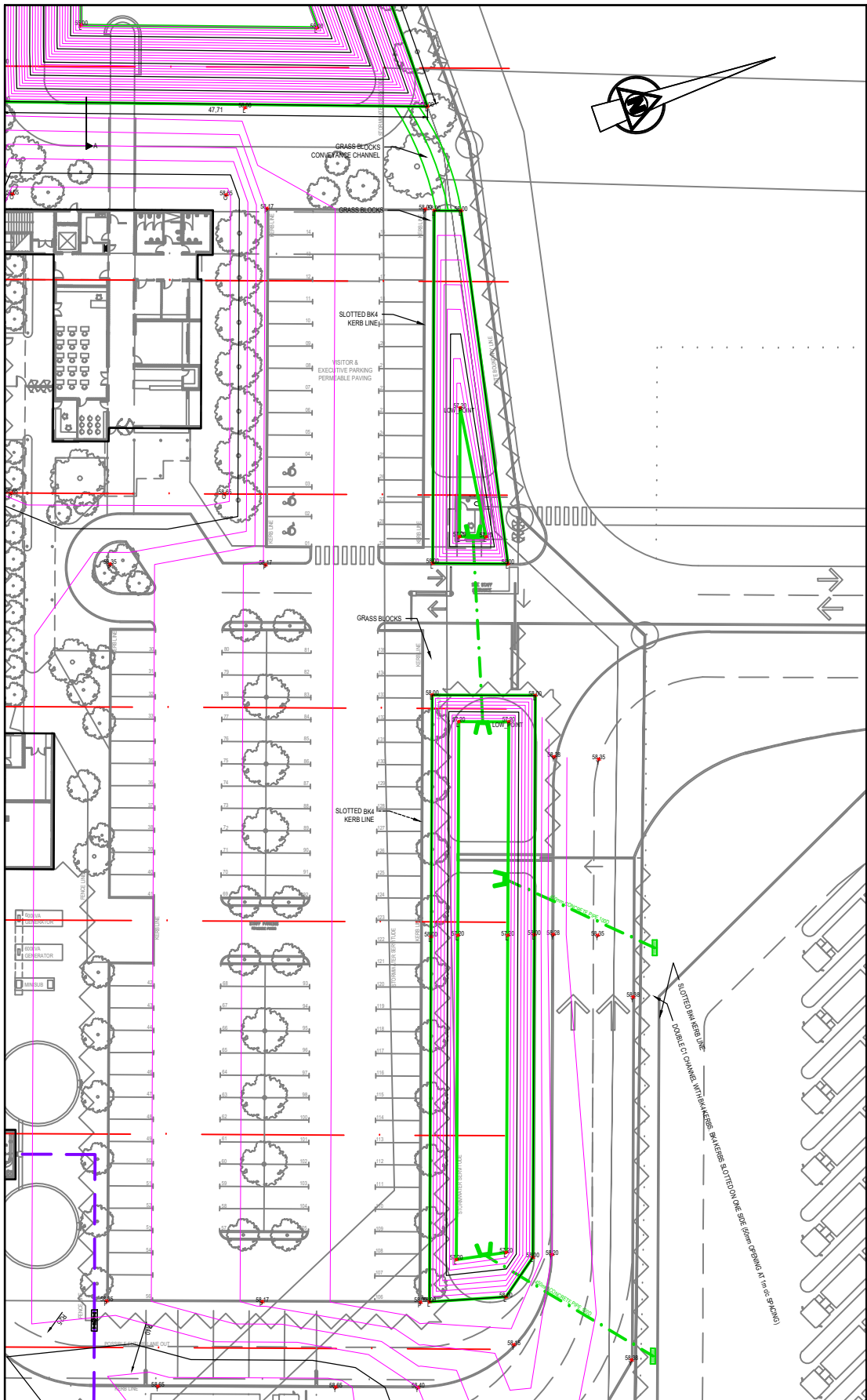


SECTION B - B

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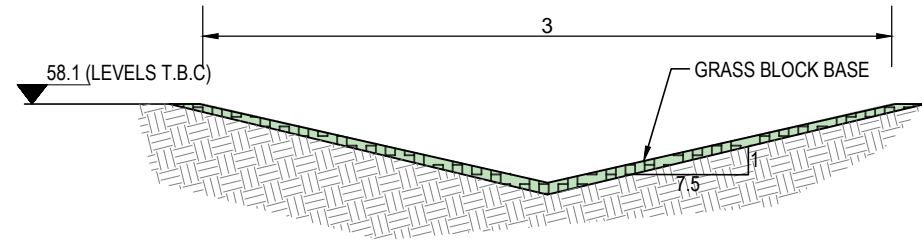


NOT TO SCALE



SWALES - TYPICAL SECTION
SCALE: 1:200

NOTE
REFER TO LANDSCAPE ARCHITECTS DRAWING
FOR DETAILED RETENTION POND PLANTING SPECIFICATIONS



CONVEYANCE CHANNEL/OVERFLOW TYPICAL SECTION
SCALE: N.T.S

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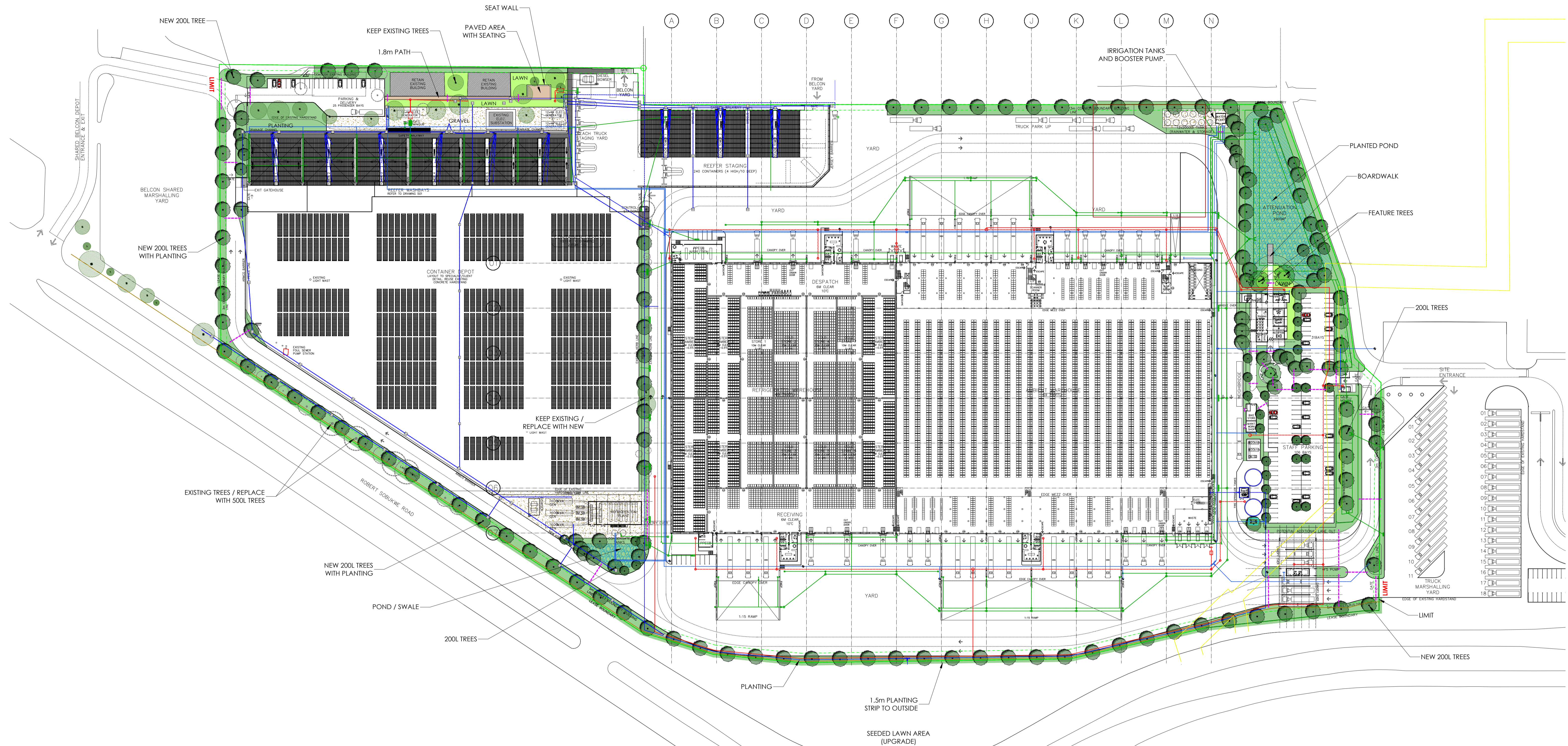
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TYPICAL DETAIL: SWALES

TEKENING Nr. 22093-511
DRAWING No.

Appendix G
Landscape Plan



CNDV LANDSCAPE ARCHITECTS' SCOPE SPECIFICALLY EXCLUDES THE FOLLOWING ITEMS:

- SURVEY WORK - BY OTHERS.
- STRUCTURAL DESIGN - BY ENGINEER.
- ARCHITECTURAL DESIGN AND DETAILING - BY ARCHITECT.
- ALL FOUNDATIONS AND STRUCTURAL DESIGN OF ALL HARD LANDSCAPE ITEMS - BY ENGINEER.
- EXCAVATIONS, RETAINING WALL DESIGN AND SAFETY ASPECTS - BY OTHERS.
- DRAINAGE, STORM WATER AND SEWER DESIGN - BY ENGINEER.
- ELECTRICAL DESIGN - BY ENGINEER.
- FIRE PROTECTION AND ESCAPE DESIGN - BY FIRE PROTECTION CONSULTANT.
- WATERPROOFING DESIGN AND INSPECTIONS - BY SPECIALIST CONSULTANT.
- ROADS, DRIVEWAYS, STATUTORY SIGNAGE AND PARKING DETAILING - BY ENGINEER.
- LAYER WORKS AND UNDER TILE SLAB DESIGN AND SPECIFICATION - BY ENGINEER.
- SLEEVES AND JUNCTION BOXES - BY ENGINEER.
- STRUCTURAL JOINTS AND EXPANSION JOINTS - BY ENGINEER.
- PAVING, TILING, DECKING, COPINGS AND FINISHES TO BE SEALED AND INSTALLED TO MANUFACTURERS' SPECIFICATION.
- POOL DETAILED DESIGN - BY SPECIALIST POOL CONTRACTOR.
- ALL WATER METERS AND CONNECTIONS - BY ENGINEER.
- PUMPS AND FILTERS AND WATER FEATURE DETAILED DESIGN - BY SPECIALIST.
- ALL WORK TO BE DONE TO COMPLY TO RELEVANT AND APPLICABLE SANS CODES.
- SHOULD ANY DESIGN WORK NOT COMPLY WITH SANS, THE CONTRACTOR SHALL INFORM CNDV IN WRITING PRIOR TO INSTALLATION FOR RECTIFICATION.
- WORKSHOP DRAWINGS TO BE PROVIDED BY SPECIALIST MANUFACTURERS, PRIOR TO MANUFACTURE. FOR ALL METAL WORK AND SPECIALIST ITEMS, TO BE SIGNED OFF BY ARCHITECT AND LANDSCAPE ARCHITECT.
- SHOULD THERE BE ANY DISCREPANCIES OR CONFUSION, ALL WORK SHOULD ADHERE TO APPLICABLE SANS CODES.

NOTES:

- 1] AN IRRIGATION SYSTEM IS TO BE INSTALLED IN ALL PLANTED AREAS. SPRAYER OVERLAP TO BE 100%. ALL TREES TO HAVE IRRIGATION AS PART OF THE OVERALL SYSTEM. ALL PIPES AND FITTINGS TO BE SABS APPROVED AND TO THE LANDSCAPE ARCHITECTS APPROVAL. ALL IRRIGATION SLEEVES ARE TO BE 110mm Ø, POSITIONED 600mm BELOW FINISHED LEVEL OF ROAD KERBING OR PAVING, AND ARE TO EXTEND 600mm BEYOND EDGE OF ROAD KERBING OR PAVING. ALL SLEEVES ARE TO RUN TO THE SIDE OF TREE OPENINGS, NOT THE CENTRE.
- 2] A MIN. 40mm IRRIGATION WATER CONNECTION POINT MUST BE PROVIDED NEAR THE IRRIGATION MAINLINE
- 3] ALL TREES TO BE STAKED USING 2x50mm DIA. TANALITH TREATED POLES DRIVEN 600mm INTO THE GROUND AND TIED BY MEANS OF TWO RUBBER HOSES AND WIRE OR COMMERCIAL TREE TIES. CARE MUST BE TAKEN NOT TO DAMAGE THE ROOT BALL OF THE TREE.

PLANT LIST

TREES (min 100L size)

- Chionanthus foveolatus
- Conocarpus capensis
- Brabejum stellatifolium
- Ekebergia capensis
- Erythrina lysistemon
- Ficus natalensis
- Ficus rubiginosa
- Harpephyllum caffrum
- Kiggelaria africana
- Nuxia floribunda
- Ropanea melanophloea
- Syzgium guineense
- Salix mucronata

SHRUBS 3-5m² (min 6 pack size)

- Agapanthus africanus blue/purple
- Aristea major
- Barleria obtusa 'Blue'
- Bauhinia galpinii
- Carissa macrocarpa 'Green Carpet'
- Cliffortia heterophylla
- Coleonema album
- Celeonema pulchellum
- Dietes grandiflora
- Elegia tectorum 'Fishoek'
- Elegia tectorum (large)
- Eriocapulus africanus
- Felicia ammeloides
- Felicia filifolia
- Leonotis leonurus - white
- Metalsia muricata
- Myrsine africana
- Pelargonium sp.
- Plectranthus ecklonii 'Medley Wood' blue
- Plumbago auriculata - blue
- Podalyria calyptrate
- Podalyria sericea
- Salvia sp.
- Scabiosa africana
- Searsia sp.
- Tecoma capensis 'Pink Bush'

GROUNDCOVERS 5-7m²

- Aptenia cordifolia 'purple'
- Agapanthus nana 'Peter Pan' / miniature blue
- Agathosma sp.
- Arctotis White
- Arctotis Pink
- Aristea africana
- Asparagus densiflorus 'Mazeppa'
- Barleria repens 'Purple Prince'
- Crassula pelucida
- Geranium incanum
- Gazania - white
- Gazania - pink
- Helichrysum teretifolium
- Juncus lomatosphyllus
- Lampranthus spectabilis 'Purple'
- Limonium perezii
- Osteospermum ecklonii
- Osteospermum jucundum
- Pelargonium capitatum
- Plectranthus ciliatus
- Plectranthus peltatum
- Plectranthus neoehlius
- Sutera cordata 'Blizzard'

AQUATIC AND MARGINAL PLANTS 3-5m²

- Apogongeton distachyos
- Bulboschoenus maritimus
- Calopsis paniculata
- Carex clavata
- Cyperus textilis
- Elegia sp.
- Ficinia nodosa
- Isolepis prolifer
- Juncus sp.
- Mariscus thunbergii
- Nymphaea capensis - blue
- Oxalis sp.
- Potamogeton pectinatus
- Prionium serratum (Palmiet)
- Restio fusticiformis
- Schoenoplectus sp.
- Scirpus nodosus
- Sparaxis bulbifera
- Spiloxene sp.
- Thymochortus insignis
- Wachendorfia paniculata
- Zantedeschia aethiopica

LEGEND:

- PROPOSED NEW TREES
- NEW FEATURE TREES
- EXISTING TREES TO REMAIN
- TREES TO BE REMOVED
- NEW PLANTING
- PAVING
- LAWN
- GRAVEL
- PONDS
- SWALE
- SLEEVES BY OTHERS

REVISIONS		
REV NO.	DESCRIPTION	TYPE OF ISSUE
00	DRAWING CREATED	FOR SUB.
01	LANDSCAPE REMOVED FROM N.E CORNER OF WAREH.	FOR SUB.
02	SWALES REVISED	FOR INFO.
03	SITEPLAN REVISED	FOR INFO.

FOR INFORMATION

BELCON LOGISTICS HUB LANDSCAPE PLAN

Drawing No: 2976 - 03 - LP - REV 03

Scale: 1:1000 @ A1 Date: 01 JUN 2023

Drawn by: MC Checked by: TdV

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